

FINAL REPORT

Long-term Environmental Management Plan

Platypus Site, High Street, North Sydney NSW

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1 Introduction

This Long-term Environmental Management Plan (EMP) has been prepared to set out the requirements for the management of the presence and potential presence of contaminated soils and groundwater in the sub-surface environment at the property known as the Platypus Site, located at 118 High Street, North Sydney NSW (the Site). The location of the Site is shown on Figure 1 in Appendix A and the layout and boundaries of the Site to which this EMP applies are provided on Figure 1. The layout and nomenclature for the current buildings and areas referred to in this EMP is provided on Figure 2.

The Site has had a diverse history as manufactured gas works plant, a Naval torpedo maintenance facility and the HMAS Platypus submarine base. Various stages of redevelopment, refurbishment and remediation works have been undertaken on the Site since 2012 to convert the Site for public open space and recreational uses as well as for light commercial and retail businesses. Remediation works that have been completed on the Site since 2012 have resulted in the retention of contaminated soils and groundwater in the sub-surface of the Site and given the continued presence of these materials in the sub-surface, the EMP has been prepared and is required to be implemented to ensure that the Site remains suitable for these uses

This EMP has been developed with reference to the relevant requirements of the NSW Environment Protection Authority (2020) *Contaminated Land Guidelines Consultants Reporting on Contaminated Land*, the National Environment Protection Council (NEPC) (2013) *National Environment Protection Measures (NEPM)*, the NSW EPA (2017) *Guidelines for the NSW Site Auditor Scheme – 3rd Edition*, the relevant requirements of the *Work Health and Safety Regulation 2017* (WHS Regulation) and on the information presented in the following reports:

- Coffey Environments Australia Pty Ltd 'Site Validation Report, Platypus Remediation Project Stage 2 Remediation Works' dated 19 January 2017 (the Validation Report);
- CONSARA Pty Ltd. 'Closure Report for Remediation and Validation Works, Platypus Site, High Street North Sydney' dated 7 July 2017 (the Closure Report);
- CONSARA Pty Ltd. 'Addendum to Closure Report for Remediation and Validation Works, Platypus Site, High Street North Sydney' dated 30 April 2018 (the Addendum to Closure Report);
- CONSARA Pty Ltd. 'Validation Report for Guardhouse Site, Platypus Site High Street North Sydney NSW' dated 4 December 2019 (the Guardhouse Validation Report); and
- CONSARA Pty Ltd. 'Validation Report for Torpedo Factory Renewal Project, Sub Base Platypus Site High Street North Sydney NSW' dated 4 June 2024 (the Torpedo Factory Renewal Validation Report);

This version of the EMP has been prepared to supersede the previous EMP dated 22 April 2020. This update to the EMP was required due to works completed in early 2024 to effect a change in the use and surface coverings of the southern area of the Site that had been occupied by the former Torpedo Factory Building.

The implementation of this EMP will be undertaken by the Sydney Harbour Federation Trust (the Harbour Trust) which is a self-funded Federal Government Agency instituted under the Sydney Harbour Federation Trust Act 2001 (SHFT Act) that has statutory responsibility for the management of the Site. This EMP and its implementation is a requirement of the Harbour Trust's Comprehensive Plan (as amended in May 2009) which was made and is being implemented in accordance with Part 5 of the SHFT Act, and the detailed precinct Site Management Plan which was approved on 8 December 2016, with Amendment 1 to this plan approved on 12 May 2021, in accordance with Section 11 of the Comprehensive Plan. Together, these two Harbour Trust plans constitute the 'Site Management Plan' for the Site under which the implementation of this EMP by the Harbour Trust is required (Part 5 of SHFT Act). This EMP is made publicly available on the Harbour Trust website as part of compliance with Part 5 of the SHFT Act and at the time of issue of this EMP can be found under the publications tab at harbourtrust.gov.au.

In addition the *Permanent Works Operation and Maintenance Manual*, (O&M Manual) for the Site, prepared by Thiess Services Pty Ltd and dated 2015 provides information regarding the operation and maintenance of the sub-



surface drainage and seepage systems and equipment installed on the Site and is required to be implemented in conjunction with this EMP.

It is noted that within this EMP direct reference is made to specific guidelines, regulations and legislation that were in force at the time of preparation of this EMP. Over time it is likely that these references will be superseded by updated and revised versions. In applying this EMP those responsible must ensure that the relevant guidelines, regulations and legislation that are applied are those that are in force at that time.

1.1 Purpose of the EMP

This EMP was prepared to document procedures that are required to be implemented to manage identified risks to users of the Site to ensure that:

- The hardstand and other surface coverings present on the Site are maintained to ensure that users of the Site are not exposed to potentially contaminated soils and/or groundwater located beneath these surface coverings; and
- Works that require disturbance of ground surface coverings on the Site, particularly in the northern area of the Site, are undertaken in a manner that protects the health of the users of the Site, being the local community and general public and any Harbour Trust Staff, contractors or workers on the Site.

It is noted that the implementation of this EMP requires the avoidance, to the extent practicable, of the undertaking of works that will penetrate the surface coverings on the Site.

The principal elements of this EMP are to:

- Detail the sub-surface conditions of the Site at the time of preparation of this EMP;
- Detail the surface conditions of the Site at the time of preparation of this EMP and detail the measures to be undertaken to ensure the integrity of these surfaces;
- Assign responsibilities for implementation of this EMP;
- Protect the health of users of the Site by ensuring maintenance of the surface coverings on the Site to prevent exposure to contaminants that may be present in the sub-surface environment; and
- Protect the health of maintenance workers on the Site if the surface coverings are disturbed by works.

This EMP sets out that the regular ongoing use of the Site for open space and recreational purposes and commercial uses requires only minimal measures, defined as 'passive' measures, be implemented, such as routine inspections and, where required rectification works, to ensure the ongoing integrity of the surface coverings on the Site. Where works are required to be undertaken that disturb those surface coverings, then this EMP sets out the measures that need to be implemented to be protective of workers undertaking those works and other users of the Site. The responsibilities for implementation of the EMP are further set out in Section 3.3 and at Table 1.



1.2 Terms used in this EMP

The Terms used in this EMP to refer to specific areas or installed features on the Site are defined below:

- **Cap** means surface treatments or finishes that are present on the Site that provide a suitable physical barrier between users of the Site and the underlying contaminated/potentially contaminated soil and groundwaters;
- **Surface Coverings** means surface treatments or finishes that form the Cap or part of the Cap as follows:
 - o <u>Permeable</u> Turf, planted beds, trees, mulch or wood chips, sandstone mulch;

OR

- o <u>Impermeable</u>
 - Hardstand surfaces, such as concrete, asphalt, pavers, stones, decogravel, seating structures, walls and their associated sub-grade materials that cannot be removed by users in the course of expected use of these areas; and
 - Building footprints.
- **Marker Layer** High visibility geotextile fabric, rigid mesh or HDPE Plastic liner installed directly onto retained fill materials forming part of the Cap in specific areas of the Site (refer to Section 2.3.3 of this EMP)
- Geosynthetic Clay Liner (GCL) Single liner installed directly onto retained compacted contaminated materials – 10-20 mm thickness – forming part of the Cap in the Northern Remediation Containment Area (refer to Section 2.3.1 of this EMP). The GCL is a high strength low permeability material made from polypropylene geotextiles and sodium bentonite powder and forms the primary barrier between the retained stabilised materials and the overlying surface treatments.
- **Minor works** works that require minimal disturbance of the surface coverings and comprise activities described in Section 3.2 of this EMP
- **Major works** works that require larger scale disturbance to the surface coverings and the underlying retained contaminated materials and comprise activities described in Section 3.2 of this EMP.
- **Control measures** measures required to be implemented during the completion of Minor Works or Major Works to protect health of workers undertaking those works, users of the Site, the surrounding community and the environment. Control measures are set out in Table 3 of this EMP.
- Northern Remediation Area the northern section of the Site in which the former gasworks infrastructure was primarily located and that was the focus of the remediation works completed as shown on Figure 3 provided in Appendix A. The final layout and survey plan of this area is provided in Appendix B and its location is also shown on Figure 2 of this EMP.
- Northern Remediation Containment Area Area within the Northern Remediation Area in which stabilised contaminated fill and soils were backfilled, compacted and covered with a series of materials that comprise the cap the extent of which is shown on the survey plan provided in Appendix C.
- **Guardhouse Site** the former Guardhouse Building and the immediate surrounding area located at entrance to the Site from High Street that is occupied for residential purposes and for use as the Harbour Trust Rangers Office. The Guardhouse Site is not subject to this EMP. The location of the Guardhouse Site is shown on Figure 2.



1.3 Structure of this EMP

The structure of this EMP is as follows:

- Section 2 Site Conditions Description of the Site, including a description of the areas and contaminants of concern that have been retained in the sub-surface of the Site.
- Section 3 Implementation of EMP Description of how the EMP is to be implemented, the works required and the persons responsible and records that need to be maintained. The primary work required is to maintain the existing surface coverings. When works are to be undertaken that disturb the surface coverings, they are defined as either Minor Works or Major Works depending on the nature of the disturbance. Examples of minor works and major works are provided;
- Section 4 Risk Control Measures Description of the different contaminants present and the risks to human health and the environment from those contaminants. Control measures are then provided that need to be implemented to ensure maintenance of existing coverings and during either Minor Works or Major Works on the Site to ensure the protection of human health and the environment during those works and to ensure that the surface coverings remain or are appropriately reinstated.



2 Site Conditions

2.1 Site Identification

The Site is located at 118 High Street, North Sydney, within the North Sydney local government area. The main access to the Site is from High Street, with a secondary access to an upper car-park area via Kiara Close.

The Site is located on land owned by the Harbour Trust identified as DP109583 (Lot A). The land adjoining the Site to the east is referred to as Harbour Land and is comprises a wharf over Neutral Bay which is owned by the NSW Government.

The area of the Site to which this EMP applies is provided on Figure 1 and the layout and nomenclature for the buildings and areas referred to in this EMP is provided on Figure 2. It is noted that the area of the wharf and the area occupied by the Guardhouse Building that has a residential unit and the Rangers office is not subject to this EMP.

The Site is bound by:

- Adderstone Avenue and residential properties to the north;
- A wharf and Neutral Bay to the east;
- Residential properties, including the lora development then High Street to the west; and
- High Street, residential properties and Kesterton Park to the south.

2.2 Site History

The Site has been utilised for various purposes since European settlement in the late 1700s, with it's earliest recorded use being associated with whaling allotments from about 1828 and as a manufactured gaswork plant between 1876 and 1932. The Australian Federal Government resumed the Site in 1942 when it was used for torpedo maintenance workshops and thein in 1967 as submarine base known as HMAS Platypus.

In 1999 HMAS Platypus including the torpedo workshops were closed and the Site was mothballed whilst the Department of Defence considered remediation and development plans. Investigations commissioned initially by Defence in the 1990s identified the presence of significant soil and groundwater contamination primarily as a results of the former gasworks operations on the Site.

2.2.1 Site-wide Remediation- 2011 to 2016

In 2005 the Site was transferred to the Harbour Trust for remediation and rehabilitation to allow for a mix of public open space and light commercial uses. Over the next few years the Harbour Trust undertook numerous stages of detailed investigations to inform the development of a plan of remediation for the whole Site. Remediation works were subsequently commissioned by the Harbour Trust in 2011 and were conducted over three stages:

- Stage 1 Preparatory works including hazardous materials removal or stabilisation in all buildings, demolition
 of the administration building (Building No. 8 see Figure 2 in Appendix A) and Store (Building No. 7- see
 Figure 2 in Appendix A) and wharf repairs;
- Stage 2 Major remediation works comprised of the following:
 - On-site ex-situ stabilisation treatment of the most significantly contaminated materials such as pure coal tars and coal tar contaminated soils from the Northern Remediation Area for off-site disposal to landfill;
 - On-site ex-situ stabilisation of other less contaminated soils and bedrock from the Northern Remediation Area and placement of these stabilised materials into on-site containment area within the Northern Remediation Containment Area that was covered with a Cap;



- Other ancillary remediation works included installation of a jet-grout cut off wall along the lower eastern boundary of the Site (alignment shown in Appendix D), in-situ stabilisation of soil materials beneath the former retort house (Building No. 11 see Figure 2 in Appendix A and plans in Appendix D), containment of existing contaminated and potentially contaminated soils beneath existing buildings and paved surfaces and construction of a seepage collection system at the base of the cut sandstone walls on Site;
- Stage 3– Landscaping works comprised the following:
 - Placement of layers of clean materials, including sands, topsoils, mulch, gravels and concrete to form the finished surfaces of the Cap across the Northern Remediation Containment Area and the Northern Remediation Area that was subject to the major remediation works;
 - o Installation of shallow drainage and irrigation systems; and
 - Planting of mass planting beds, trees and grasses.

These works commenced in 2011 and were completed in mid 2016. Comprehensive details of these works are provided in the Validation Report and Closure Report. In addition, the O&M Manual provides information regarding the operation and maintenance of the sub-surface drainage and seepage systems and equipment installed during the remediation works and is required to be implemented in conjunction with this EMP. In December 2017, NSW EPA Accredited Site Auditor, Mr Graeme Nyland, issued a non-statutory Site Audit Statement and Site Audit Report that certified that the remediation works had been completed and that the Site was suitable for open space/recreational and commercial/industrial uses subject to the implementation of this EMP.

2.2.2 Remediation of Guardhouse Site for Residential Land Use -2019

In 2019 the Harbour Trust undertook the repurposing of the former Guardhouse Building and the immediate surrounding area from its former commercial/industrial land use and into a residential leasing and as a Harbour Trust Ranger's office (the Guardhouse Site). The re-purposing or redevelopment works comprised the completion of internal improvements in the building via the conversion of the internal areas into two separate residential apartments as well as a dedicated office area for Harbour Trust Ranger's that manage access into the Site. The rear external areas have been landscaped to comprise primarily concrete and paved outdoor areas and raised garden beds with the frontage of the building at High Street to have general garden beds. The concrete slab across the carport area has also been improved via resurfacing with new concrete.

The works to the Guardhouse Site enabled a change in use from a commercial/industrial land use to a more sensitive residential land use with minimal opportunities for soil access, without the requirement for the implementation of the EMP. These works were documented in the Guardhouse Validation Report. In December 2019, NSW EPA Accredited Site Auditor, Ms Rowena Salmon, certified that the Guardhouse Site was suitable for residential land use with minimal opportunities for soil access, without the requirement for the implementation of the EMP.

2.2.3 Remediation of the RANTME Building (Building No.1), known as the former Torpedo Factory Building, for Open Space and Recreation and Commercial Land Use – 2022-2024

At the completion of the site-wide remediation works in 2016 the RANTME Building, known as Building No.1 and as the former Torpedo Factory Building, located across the southern area of the Site, was suitable for commercial/industrial use, subject to the EMP. There remained the potential for contaminated materials to be present beneath the ground floor slabs, on-ground slabs and footings of the building. Since 2016, Level 3 of the RANTME Building had been converted into a carpark and for a period of time was utilised by the Harbour Trust for their offices, with the remaining Level 1 and 2 areas vacant and unoccupied.



In 2022 the Harbour Trust commenced a project, referred to as the Former Torpedo Factory Renewal Project, to convert the building and surrounds from the existing commercial/industrial land use to a mix of open space and recreational land uses and retained existing commercial/industrial land uses (being carparking on Level 3). The works comprised the removal of the large parts of the building, including the eastern areas and the western walls, the creation of an updated carpark area on the retained areas of Level 3, new outdoor areas on the eastern part of the retained Level 3 area (and the creation of a new small indoor area being the service room and an amenities building), a new landscaped park area to the east in the footprint of the former eastern part of the building (referred to as the L1 Foreshore Park) and a new landscaped forecourt to the west at High Street (referred to as the High Street Forecourt).

Given that these works would result in a significant change to the surface coverings across the eastern and to a lesser extent across the western areas of this southern area of the Site, a remediation strategy consistent with that already adopted for the Site, being capping, containment and long-term management, was implemented as part of the works. The results of these works provide new hardstand and soft landscape surface coverings along with the retention of the Level 3 concrete slab, to ensure that they provide an effective isolation and capping layer across the Renewal Site. The capping, containment and management of the retained contaminated and potentially contaminated fill materials was undertaken after the demolition and as part of the construction phase of the works and has resulted in the southern area of the Site being made suitable for its open space and recreational and commercial/industrial land uses, subject to the implementation of an updated and revised EMP. These works were completed in March 2024 and documented in the Torpedo Factory Renewal Validation Report. This version of the EMP is the updated and revised version required at the completion of these works.

2.3 Areas and Contaminants of Concern

The remediation works completed on the Site resulted in the retention of contaminated and potentially contaminated soils and groundwaters in the sub-surface beneath various areas of the Site. The contaminants of concern primarily comprise Total Petroleum Hydrocarbons (TPHs), Benzene, Toluene, Ethylbenzene, Xylenes (BTEX), Polycyclic Aromatic Hydrocarbons (PAHs), Metals (Arsenic, Cadmium, Copper, Chromium, Nickel, Lead, Mercury and Zinc) and Asbestos.

These areas and the associated contaminants of concern are detailed below.

2.3.1 Cut-off Wall

The remediation works on the Site required the installation of an impermeable cut-off wall behind the existing seawall along the eastern boundary of the Site to contain the northern section and central section of the Site. The primary objective of the wall was to prevent the ingress of sea water from Neutral Bay and the discharge of groundwaters from the Site to Neutral Bay, whilst also protecting and stabilising the existing sea wall.

The cut-off wall was installed as a jet grout wall with a 'U-shaped' alignment around the north-eastern, eastern and south-eastern boundary of the Northern Remediation Area and then in a north-south alignment along the eastern boundary of the Site to the south of the Northern Remediation Containment Area. The as-builts of the installed jet-grout wall is provided as Appendix D of this report. The Validation Report and the Closure Report stated that the constructed jet-grout wall met the specified design criteria and acts as an impermeable hydrological barrier to the Site.

The jet-grout wall is comprised of a mixture of cement self hardening grout and the soils that were present in the areas that the wall was constructed. As such the jet grout wall itself has been formed utilising contaminated soils and as such contains some potential contaminants of concern such as PAHs, TPHs and heavy metals. It is noted that the jet-grout process serves to bind these contaminants in a stable matrix such that contaminants may only be able to be released if the jet grout is significantly disturbed or compromised.



2.3.2 Contained Stabilised Contaminated Materials – Northern Remediation Containment Area

The remediation works on the Northern Remediation Area, shown outlined in red on Figure 2 and 3 of Appendix A, resulted in the removal of all soil materials to bedrock. Whilst the most significantly contaminated materials were disposed off-site, the less contaminated materials were treated on the Site. This treatment served to stabilise the primary contaminants, PAHs and TPHs, in the soils. These stabilised soils were then reinstated into the deepest areas of the resultant excavation in compacted layers and were finished above the pre-existing surface levels to form the basis of the final landscaping works. A Cap comprised of various layers of materials was then placed over the stabilised soils. The extent of the retained stabilised materials is referred to as the Northern Remediation Containment Area as shown in plans provided in Appendix C and Appendix E. Details of the Cap are provided in detail below.

Details of Cap over Northern Remediation Containment Area

The extent of the contained stabilised contaminated materials and the cap that covers the Northern Remediation Containment Area is shown on the figures provided in plans provided in Appendix C and Appendix E. The Cap is comprised of the following (in order of placement over the retained materials):

- **Geosynthetic Clay Liner (GCL)** Single liner installed directly onto retained compacted materials 10-20 mm thickness. The GCL is a high strength low permeability material made from polypropylene geotextiles and sodium bentonite powder and forms the primary barrier between the retained stabilised materials and the overlying surface treatments. The specifications for the GCL installed on the Site are provided in Appendix F;
- Drainage blanket including a flow net Panel liners and a single layer of high-density polyethylene (HDPE) geonet installed directly over GCL 10 mm thickness. The specifications for the panel liners and geonet installed on the Site are provided in Appendix F;
- **Drainage layer** Layer comprising fine-grained, well graded, non-erodible cohesive material with stones of up to 25mm Variable thickness between 0.4 and 1.1 metres dependant on final surface covering;
- **Final surface treatments** concrete, structural sands, sub-grade materials, soils, gravels, paving, topsoils, mulch, vegetation (plants, shrubs, grass etc) Variable thickness between 0.2 and 0.5 metres dependant on final surface covering.

The registered survey of the top of the GCL and top of the drainage layer and as-builts for these features is provided in Appendix E and then the thickness of the capping layer over the top of the GCL to the finished surface levels of the Site is provided on the plan in Appendix C. The final surface levels and features across the Northern Remediation Area are generally provided in Appendix B. However, it is noted that as set out the Addendum to Closure Report, some changes have occurred to the surface features shown in Appendix B comprising the installation of staircases, additional paving, concrete and playground equipment, all of which were constructed above the finished surface RLs shown in Appendix B and that on these areas the resultant finished surfaces have higher RLs than those provided on Appendix B.

It is noted that a constructed drainage system was installed through the drainage layer and upper clean materials to divert waters and stormwaters laterally to a system of pits and pipes. Part of the as-builts for these systems is provided in Appendix G and it is noted that the full detail on this system is provided in the O&M Manual for the Site.

Cliff Line Seepage water Collection and Discharge System - Northern Remediation Containment Area

The Site has a number of expansive vertical sandstone rock cliff faces. Groundwaters present in sandstone beneath the higher areas of the Site and off-site continue to migrate to the east and discharge as seeps along the jointing, bedding planes or fractures in the sandstone rock at these faces. Concentrations of cyanide and some other metals in these seepage waters have been found to be elevated and as such a cliff line seepage water collection



system was installed to collect seepage water for diversion for collection and ultimate disposal via tradewaste to sewer.

The system comprises a continuous series of grated drains installed along the base of the cliffs with the finished ground surfaces and a drainage layer present along the north-western corner of the Northern Remediation Containment Area. The grated drainage system extends along the driveway into the Site from High Street and continues to the north along the base of the cliff that runs behind Building No.2 and continues along the base of this cliff into the Northern Remediation Containment Area. Within the Northern Remediation Area the grated drain extends to where the Cap commences in the north-west.

Along the base of the cliff line where the Contained Stablised Materials are present, a drainage layer was installed along the cliff face prior to the placement of these materials. This drainage layer consists of a plastic layer (Podrain CD18) fastened to the cliff with a water proof membrane (Emerproof HDPE) preventing water migration between the fill materials and the drainage layer. The drainage layer drains to the geofabric covered pipe which discharges to the south into the drainage system.

This system ultimately discharges seepage waters into a collection tank in the sub-surface of the Northern Remediation Containment Area for disposal to sewer under a Tradewaste Permit. The as-builts for this systems is provided in Appendix G and it is noted that the full detail on this system is provided in the O&M Manual for the Site.

2.3.3 Retained Contaminated Materials Southern Area

The works completed to convert the RANTME Building (known as the former Torpedo Factory Building) located on the southern area of the Site (as shown on Figure 2) to a new public domain and publicly accessible open space and recreational areas in the east and west of the retained and opened up Level 3 of the building has resulted in the retention of contaminated soils/fill materials in the sub-surface above bedrock and beneath pre-existing or newly installed surface coverings to form a capping layer across this area. The Cap is comprised of the following (in order of placement over the retained materials):

- Impermeable surface coverings comprised of concrete, paving and decogravel pavements;
- Permeable surface coverings on the soft landscaped planted beds which are comprised of the following:
 - Marker Layer– High visibility geotextile fabric, rigid mesh or HDPE Plastic liner installed directly onto retained fill materials. The specifications for the marker layer installed in this southern area of the Site are provided in Appendix I. The extent of the marker layer placed in this area of the Site and the RLs of the top of the marker layer is provided on Figures 3 to 7 of the Torpedo Factory Renewal Validation Report which are included in Appendix I.
 - Landscaping Materials Growing medium soils and mulch to thickness of 300 mm over the marker layer in areas of shallow planting and to thickness of 700 mm over the marker layer in areas of deep planting such as trees. Further, the sub-soil drainage channels installed in the L1 Foreshore Park on the eastern area have been lined with HDPE plastic and then the slotted drainage pipes installed and covered with drainage aggregates and the covered with sandstone mulches. The thickness of the materials placed in these areas of the Site above the marker layer and the RLs of the finished surfaces in these areas is provided on Figure 5 to 7 of the Torpedo Factory Renewal Validation Report which are included in Appendix I.

It is noted that on the eastern lower area where the L1 Foreshore Park is present, the existing sandstone seawall that runs along the eastern edge of the cliff line remains present and that the area between this seawall and the permanent fenceline of the park, referred to and shown in Figure 2 as the Seawall Protection Zone (see also Figure 4 of the Torpedo Factory Renewal Validation Report in Appendix I), has a minimal Cap comprising of a Marker Layer of high visibility rigid mesh that is covered with minimal thickness landscaping comprised of approximately 70 mm thick sandstone mulch and shallow planting. Access to this area is not permitted for any other purpose except for landscape maintenance by landscape workers.



It is noted that for many of the large planter beds and boxes on the eastern and western areas of the retained areas of Level 3 which contain planting, are present on the concrete slab, which provided the Cap in these areas.

The registered survey and as-builts for these features for this southern area of the Site is provided in Appendix I and the location of the L1 Foreshore Park is shown on Figure 2.

2.3.4 Retained Contaminated Materials Remaining Areas of Site

In various areas across the Site, outside of the Northern Remediation Area, there remains the actual or potential for contaminated soils and/or groundwaters to have been retained in the sub-surface above bedrock and beneath pre-existing or newly installed surface coverings. A summary of these areas is provided below:

• Driveway Area between the Workshops (Building No.10) and the Submarine School (Building No.2) (as seen in Figure 1 and 2 and in Figure 10 of Appendix A) – The driveway that extends from High Street in an L-shape to the southern boundary of the Northern Remediation Containment Area was subject to excavation to depths of 1.2 metres below the ground surface (m bgs) or to/into sandstone bedrock, whichever was first encountered.

Whilst the excavation for the driveway from High Street down to between the RANTME Factory (Building No.1 and also referred to as the former Torpedo Factory Building) and the Administration Building (Building No.3) was completed into bedrock in the base and walls, between the Workshops (Building No.10) and the Submarine School (Building No.2) there were areas of the driveway excavation where soils materials were retained at depths greater than 1.2 m bgs. In this area a marker liner comprised of orange HDPE plastic was installed along the base of the excavation prior to backfilling. This liner was installed to alert future users who may access depths greater than 1.2 m bgs that potentially contaminated materials are retained beneath the liner.

In order to prevent undermining of the buildings on either side, the excavation along the roadway was only able to extend laterally to the edges of the buildings. During these works it was observed (reported in Validation Report) that the soil materials retained in the walls and beneath the buildings contained fragments of asbestos containing materials. No marker liner was installed along the face of the walls.

 Workshops (Building No.10), Submarine School (Building No.2), Coal Bunker House (Building No.13) and Compressor House (Building No. 12) (as shown on Figure 2 and in Figure 2 of Appendix A) – These buildings were subject to various remedial works, including excavation and removal of contaminated materials present in the floors or sub-floors and, where practicable, re-surfacing with concrete. The final surfaces of these areas is shown on Figure 13 of Appendix A. However, there remains the potential for contaminated materials to have been retained beneath the ground floor slabs, retained on-ground slabs and footings and/or for hazardous building materials to be present.



3 IMPLEMENTATION OF THIS EMP

This EMP is required to be implemented to ensure:

- Maintenance of the surface coverings of the Site in the condition described in this EMP; and
- Works that require the disturbance of the surface coverings, described below as 'minor works' and 'major works', are conducted in accordance with the requirements set out in this EMP.

It is noted that the implementation of this EMP requires the avoidance, to the extent practicable, of the undertaking of works that will penetrate the surface coverings.

3.1 Maintenance of Surface Coverings

For the purpose of this EMP, the surface covering of the Site is defined as the pavements, roadways or slabs constructed of concrete, bitumen, asphaltic cement or other materials such as bricks, paving stones, growing mediums (topsoils, mulch or similar), vegetation etc, that cannot be removed readily without the use of tools.

The program of the maintenance of surface coverings is required for all areas of the Site, however, frequency of inspections and timeframes for repair works differs depending on the nature of the surface covering.

3.1.1 Impermeable Surface Cover Maintenace

The integrity hardstand surfaces on the Site (comprised primarily of concrete and/or bitumen, paving, decogravels, walls or covered by building footprints) must be maintained to prevent contact with the underlying potentially contaminated soils and groundwater.

Inspections must be undertaken to ensure that the impermeable surface coverings are not showing signs of deterioration. Inspections must be undertaken at least once per every 12 months. These inspections are to be undertaken as a walkover and can be completed by Harbour Trust staff or contractors as part of routine, specific or general works.

Impermeable surface coverings are present across the Site in both indoor (in an enclosed space that comprises walls and roofing, such as a building or similar) and outdoor (open outdoor areas or where good ventilation is present at all times) areas. In both indoor and outdoor areas, where practicable, cracks or gaps within the hardstand pavement are to be no wider than 250 mm at the widest point.

Should a pothole or cracking wider than 250 mm be observed in the hardstand pavement, such that workers or users of this area could be practicably exposed to the underlying sub-surface materials present beneath the pavement and its associated sub-grade, nominally at depths greater than 0.2 metres (m) from the surface, (such as through dermal (skin) contact due to requirements to handle sediments or silts generated from such areas during weather events or similar) then repairs are required to be undertaken following the protocol set out below for minor or major works as appropriate..

Should a pothole or cracking area wider than 250 mm be observed in the hardstand, such that workers or users of this area could be practicably exposed to the underlying sub-surface materials present beneath the impermeable surface coverings or marker layer (such as through dermal (skin) contact) then repairs are required to be undertaken following the protocol set out below for minor or major works as appropriate. At the time that such compromises of the integrity of the impermeable surface coverings are identified an exclusion zone must be established, by cordoning off using physical barriers such as bollards, high visibility mesh, fencing or similar, to restrict access to the impacted area until repairs can be undertaken.

Guidance on determining the period of time in which repairs must be made is as follows:

 Indoor Areas which are in regularly used and occupied by workers or users of the Site (For example - utilised at least twice per week for a work period of at least 4 hrs continuous each time or greater) – Repairs to hardstands must be made within a 4 week period;



- Indoor Areas which are not regularly used or occupied by workers or users of the Site (For example utilised once to twice per week for a work period of up to 4 hrs continuous each time or less) – Repairs to hardstands must be made within a 10 week period);
- Outdoor Areas which are regularly used and occupied by workers of users of the Site (such as driveways, access walkways, entrances to buildings) Repairs to hardstands must be made within an 8 week period;
- Outdoor Areas which are not regularly used or occupied by workers or users of the Site Repairs to hardstands to be made within a timeframe considered to be reasonable by the Harbour Trust but no longer than a 4 month period.

3.1.2 Permeable Surface Cover Maintenance

The areas that are covered with permeable surface covers such as turf, plantings, trees or mulch or wood chips or similar must be maintained to ensure the integrity of these surface coverings and to prevent contact with the underlying retained contaminated fill materials and groundwater. Inspections must be undertaken to ensure that no large-scale erosion or removal of the surface coverings occurs that exposes the materials present beneath the surface coverings such that the marker layer (where present) can be visually observed and/or retained contaminated fill materials are exposed.

Inspections must also be undertaken to ensure that no large scale erosion occurs such that deeper potentially contaminated soils are exposed. Inspections must be undertaken at least once per every 6 months. These inspections are to be undertaken as a walkover and can be completed by Harbour Trust staff or contractors as part of routine, specific or general works. It is noted that for the Seawall Protection Zone on the southern area of the Site as shown on Figure 2, walkovers are not required as visual inspections to assess integrity of surface coverings can be completed from the fenceline that forms it boundary with the L1 Foreshore Park.

Should such erosion or similar be observed or should the marker layer (where present) or the retained fill materials be observed or exposed then repair works are to be undertaken. At the time that such compromises of the integrity of the permeable surface coverings are identified an exclusion zone must be established, by cordoning off using physical barriers such as bollards, high visibility mesh, fencing or similar, to restrict access to the impacted area until repairs can be undertaken. If required, the exclusion zone is to be formed by cordoning off using physical barriers such as bollards, high visibility mesh, fencing or similar, to restrict access to the general public. Repairs to re-establish the surface covering are required to be undertaken within 4 weeks. Inspections must be undertaken to ensure that the repair works has been appropriately completed prior to users being able to re-gain access to these areas.

3.2 Disturbance of the Surface Coverings – Minor Works and Major Works

For the purpose of this EMP:

- **Minor works** comprise works that require minimal disturbance of the surface coverings and comprise activities such as:
 - Hardstand or pavement maintenance works at surface level only such as filling in of cracks, patching of holes or small scale replacement of sections of pavement where sub-surface retained soils are not required to be disturbed;
 - Landscaping works on permeable surface coverings installed as part of Cap on sub-surface retained soils such as mowing, shallow planting, mulching, raking leaves, maintenance of gravel pathways or similar, where direct contact with the materials used in the Cap is possible, but no contact with the sub-surface retained soils (soils greater than 0.2 m depth) or contact with the GCL (described in Section 2.3.2) or Marker Layer (described in Section 2.3.3) is possible.
 - Landscaping works on planter beds installed on impermeable surface coverings (planter beds with concrete base no contact with underlying retained soils). This includes any landscaping works on these planter



beds. These planter beds are aboveground and are located in areas in the atrium of the Workshop Building (Building No. 10); to the south of the Workshop Building (Building No. 10); the eastern area of Level 3 of the RANTME Building and in some locations in High St Forecourt (see Figure 5 of Appendix I).

- **Major works** comprise larger scale disturbance to the surface coverings and the underlying materials and comprise activities such as:
 - Any works during which the impermeable surface coverings are disturbed such that direct contact with the underlying retained contaminated fill material or groundwater is possible or will occur;
 - Any works during which the surface covering is disturbed such that the Marker Layer/GCL (where present) is required/potentially required to be exposed and/or penetrated and where direct contact with the underlying retained contaminated fill material or groundwater is possible or will occur;
 - Any works across the Northern Remediation Containment Area during which direct contact with the subsurface soils and/or groundwater at depths greater than 0.2 m is possible;
 - o Any works that directly affect or disturb the Cut-off Wall;
 - Construction and maintenance of sub-surface services, such as gas, electricity, stormwater, surface drainage, telephone, cabling and water supply;
 - Installation of equipment or undertaking of construction that requires excavation of the surface and subsurface where direct contact with the retained materials is possible;
 - Installation and maintenance of features or equipment that require disturbance and/or disposal of the surface and/or sub-surface soils where direct contact with the sub-surface soils or groundwater is possible.

The control measures required to be implemented during the completion of Minor Works or Major Works are provided in further detail in Section 4.3.

3.3 Responsibilities

The Harbour Trust, as the Site Owner, is responsible for the overall implementation and maintenance of this EMP and for ensuring that owners, occupiers, tenants and contractors working on the Site have been informed of the requirements of the EMP prior to commencement of works.

It is the responsibility of the Harbour Trust to avoid, to the extent practicable, the undertaking of works on the Site that will penetrate the surface coverings and underlying marker layer (where present) and potentially expose the underlying retained contaminated materials (Major works).

The supervisor or person-in-charge of works on the Site is responsible for implementing the requirements of the EMP during the planning process for any works to be conducted, during the course of any works that fall into the "Minor" or "Major" categories as defined below, and at the completion of any "Minor" or "Major" works. The specific responsibilities of the Harbour Trust and the supervisor or person in-charge of the works are outlined in Table 1.



Table 1: Roles and Responsibilities

Position and Company/Entity	Responsibilities
The Site Owner (Harbour Trust OR nominated	 Advise persons occupying and working at the Site of the requirements of the EMP;
	Ensure implementation of the EMP requirements on the Site;
	 Ensure appropriate consents and licences (as required) are obtained for the works;
	 Procure the training and induction of employees and contractors before and during the works, as appropriate and relevant;
	• Provide a copy of the EMP to the occupiers, supervisor or person-in-charge of occupier/tenant employees and/or contractor/s who are undertaking the works;
	• Ensure relevant and appropriate project/occupier/tenant staff and contractors comply with the requirements of the EMP;
	• Ensure relevant and appropriate project/occupier/tenant staff and contractors clearly understand the requirements of the EMP and ensure that compliance with the EMP is a condition of any agreement with these parties;
	• Ensure the conditions of the EMP are implemented and supplemented, if necessary, by conditions of any relevant planning consent;
	 Obtain advice if the conditions on the Site are changed, and, if necessary, arrange for an appropriately qualified person to update the EMP, informing other relevant and appropriate parties, including tenants, of the changes;
	• Ensure the Site is maintained in accordance with the EMP;
	Ensure corrective action is undertaken where verified complaints are made;
	 Provide the EMP for inclusion on the relevant records maintained by project/occupier/tenant.



Position and	Responsibilities		
Company/Entity			
Supervisor or person in charge of works	Implement the EMP to ensure compliance;		
(Owner/Occupier/Tenant/	• Complete the registers, databases and records required by the EMP;		
Sub-	Conduct works in an environmentally responsible manner;		
Contractor/Environmenta I Consultant)	 Meet relevant Work and Occupational Health and Safety regulatory requirements; 		
	 Implement the works in a safe and responsible manner; 		
	 Notify the Site Owner if suspected contaminated fill materials is encountered during works on the Site; 		
	 Complete non-conformance and corrective action reports as required and undertake follow-up corrective actions, as required; 		
	 Conduct groundwater monitoring as required in the EMP; 		
	• Undertake audits of activities in accordance with the requirements of the EMP;		
	Ensure non-conformance and/or complaints are reported to the Site Owner		
	 Undertake corrective actions in response to requests made by the Site Owner regarding specific environmental or safety issues; 		
	Ensure all works comply with relevant regulatory requirements;		
	• Inform the Site Owner if conditions change significantly from those documented in the EMP.		

3.4 Record of Implementation of the EMP

Records of the implementation of this EMP must completed and maintained by the Site Owner and include, but may not be limited to, the following:

- EMP Induction Register Register of persons inducted under the EMP. This register must include the name of the person, their employer, the date of induction, the nature of works being completed, whom the inductee is working on behalf of, the person that provided the induction and signatures of both the instructor and the inductee;
- Inspection Records Records of inspections of the integrity of the Cap across the Site as set out in Section 4.3;
- Asbestos Materials Handling or Removal Records Records of any works requiring the handling and removal of asbestos or other hazardous building materials as set out in Section 4.4;
- Waste Disposal Records Records of any works requiring the off-site disposal of excavated soils or similar as set out in Section 4.5;
- Unexpected Finds Register Records of any unexpected finds as set out in Section 4.6; and
- Complaints Register Records of complaints and response works as set out in Section 4.7.

These records must be maintained by the Site Owner such that they are available to any external auditor for review on request.



3.5 Document Revision

This EMP is required to be reviewed if a change of use or redevelopment of the Site occurs that results in a material change to the surface coverings on the Site or on the Cap across the Northern Remediation Containment Area or to the Cut-off Wall as described in this EMP. Any amendments or revisions or similar of the EMP must be controlled by the Harbour Trust and be completed by a suitability qualified environmental consultant. In making any amendments or revisions the Harbour Trust must provide the appointed suitably qualified person with the reports referred to in Section 1 of this EMP.

Should the EMP be varied due to a change in use or redevelopment of the Site occurs that results in a material change to the surface coverings and/or use of the Site as described in this EMP, the revised EMP must be subject to review by a NSW EPA Accredited Site Auditor (Accreditation under the *Contaminated Land Management Act* 1997) for acceptance as appropriate and to ensure that the revised EMP continues to be able to be legally enforceable.

It is the responsibility of the Site Owner to ensure the EMP supplied to any person is the current updated or amended version.

It is the responsibility of the supervisor or person-in-charge of works proposed to be undertaken to ensure they have the current version of the EMP.

The up-to-date version of the EMP will be available from the Site Owner.

The EMP is required to be continually implemented (in perpetuity) to ensure that the Site remains suitable for use for open space recreational and commercial/industrial land uses.



4 Risk Control Measures

This section provides summary information on mechanisms whereby exposure to the contamination present in the sub-surface soils or groundwater could occur (known as exposure pathways) and guidance on precautionary control measures for activities on the Site that have the potential to result in exposure to these materials.

It should be noted, however, that this EMP provides general guidance only and does not constitute a health and safety plan, job safety analysis, risk assessment or any other considerations under the Work Health & Safety Act 2011 (WHS Act) (or relevant legislation and regulations current at the time of the proposed works).

Potential human health risks associated with the presence or potential presence of petroleum hydrocarbon, polycyclic aromatic hydrocarbon and heavy metal contaminated soils and groundwater and asbestos in soils beneath the Site are:

- Exposure to vapours, dust, soils or groundwaters generated during the completion of works that disturb the sub-surface; and
- Inappropriate handling and waste management of excavated or excess soil materials or groundwaters.

4.1 Exposure Pathways

The presence and potential presence of petroleum hydrocarbons, polycyclic aromatic hydrocarbons and some metals within the sub-surface soils and groundwater beneath the Site does not affect the present safe use of the Site under the current land use scenarios and whilst the existing surface coverings are undisturbed.

However, if these surface coverings are disturbed through Minor or Major works as defined in Section 3.2, it is possible that a risk of exposure may result. In order to develop appropriate measures to control this increased exposure, it is necessary to understand the potential exposure pathways.

A summary of the potential health affects and the exposure pathway for these potential contaminants is summarised in Table 2 below.

Contaminant of Concern	Source	Chemicals	Physiological Effect	Exposure Pathway
Petroleum Hydrocarbons (TPH), including Benzene, Toluene, Ethyl- benzene and Xylenes (BTEX)	Soils, soil vapour and groundwater present in sub-surface beneath the Cap in the Northern Remediation Containment Area and beneath buildings and pavement across remainder of Site.	TPH and BTEX	Inhalation of volatile hydrocarbons, if present, may cause central nervous system effects such as headaches, blurred vision and narcosis, when present in high concentrations. Repeated skin contact may result in allergic dermatitis and skin cancer. Skin contact may cause dermatitis. Long term or chronic exposure may result in liver damage, effects on the blood forming- systems.	Inhalation of vapours or contaminated dust and ingestion of contaminated soil/dust are the primary pathways for exposure. Dermal absorption is low to negligible

Table 2: Exposure Pathways



Contaminant of	Source	Chemicals	Physiological Effect	Exposure Pathway
Polycyclic Aromatic Hydrocarbons (PAHs) – Carcinogenic PAHs	Soils, soil vapour and groundwater present in sub-surface beneath the Cap in the Northern Remediation Containment Area and beneath buildings and pavement across remainder of Site.	PAHs, including benzo(a)pyren e	Inhalation/ingestion of PAHs (as dusts with PAHs are not very volatile) may cause bronchitis and possibly cancer of the respiratory system and different types of tumours. B(a)P is a genotoxic carcinogen. Repeated skin contact may result in allergic dermatitis and skin cancer	Inhalation of vapours or contaminated dust and ingestion of contaminated soil/dust are the primary pathways for exposure. Dermal absorption is low to negligible
Metals (Arsenic, Cadmium, Chromium, Copper, Lead, Nickel, Mercury and/or Zinc)	Soils and groundwater present beneath the Cap in the Northern Remediation Containment Area and beneath buildings and pavement across remainder of Site	Arsenic, Cadmium, Copper, Chromium, Nickel, Lead, Mercury and Zinc	Toxic to a wide range of organs and tissues, and variety of toxicological end points – reproductive toxicity, neurotoxicity, carcinogenicity	Ingestions of soil and dust is considered to be the most significant pathway of exposure for inorganics in soil. Dermal absorption is also considered but is generally considered to be negligible
Asbestos	Soils containing bonded and/or friable asbestos. It is noted that no respirable asbestos fibres have been previously identified in surface or shallow sub- surface soils on the Site.	Asbestos Containing Materials (ACM)	Inhalation of asbestos fibres can cause asbestosis, lung cancer and mesothelioma. The risk of contracting these diseases increases with the number of fibres inhaled and the risk of lung cancer from inhaling asbestos fibres is also greater if you smoke. People who get health problems from inhaling asbestos have usually been exposed to high levels of asbestos for a long time. The symptoms of these diseases do not usually appear until about 20 to 30 years after the first exposure to asbestos	Inhalation of loose/friable asbestos from disturbed soils.



4.2 Control Measures for Current Activities

Under the current land use and based on the existing surface coverings on the Site there is low to negligible risk of harm to the human health of users of the Site and to the ecological aquatic environment of Neutral Bay by the presence of materials that contain or may contain the above listed potential contaminants within the sub-surface of the Site. The surface coverings and cut-off wall are required to be maintained for the lifetime of this EMP to ensure that this low to negligible risk of exposure is maintained.

The surface coverings across the Site are required to be regularly inspected, at least once per every 12 months. It is the responsibility of the Owner to ensure that these inspections are undertaken. As a conservative measure the inspections are to be undertaken as a walkover across the surface of the Site. The inspection must make a written and photographic record of the following:

- · General condition of hardstand pavement surfaces and unsealed surfaces;
- Presence of any subsidence, cracks, openings, degradation, erosion or similar in the surface coverings the nature, extent and location needs to be recorded and rectification works are required to be implemented;
- Presence of any obvious repair/maintenance works to the surface coverings- the nature, extent and location needs to be recorded;
- Presence of any excavation works into the sub-surface and the control measures being undertaken; and
- Any other observations on the condition and/or integrity of the surface coverings.

Where rectification works are required to be implemented or where repair/maintenance works are being undertaken, the Owner must ensure that these works are undertaken in accordance with the measures set out in this EMP. On completion of such works, the Site Owner must conduct an inspection to ensure that the surface coverings have been adequately re-instated/restored.

The record of the required inspections is required to be kept and maintained by the Site Owner.

4.3 Control Measures for Minor Works and Major Works

Minor Works on any part of the Site do not require specific controls and as long as the underlying retained contaminated and potentially contaminated materials beneath the surface coverings of the Cap (including the Marker Layer/GCL where present) are not disturbed then no specific controls are required.

Where Major Works are required to be undertaken (where the Cap is disturbed such that underlying retained fill materials or Marker Layer/GLC (where present) are disturbed/exposed) additional control measures may be required.

Whilst it is not possible to assess the impacts from all future activities, it is possible to consider exposure scenarios likely to be associated with a range of general maintenance and intrusive works.

The control measures required to be implemented for "Minor Works" and "Major Works" are set out below.



Table 3: Control Measures for Minor Works and Major Works

Works	Examples of Types of Works	Possible Risks to Users	Possible Risks to the	Control Measures Required
			Environment	
MINOR WORKS				
Minor works	Impermeable (hardstand, pavement, concrete or similar)	Inhalation of	Low risk of	WHS and General Requirements
comprise activities	surface cover maintenance:	vapours or	runoff. May	-No eating, drinking, smoking; avoid contact with soil
that only require	-Filling in of cracks, patching of holes	dusts	impact	(wear gloves)
minimal	-Small scale replacement of sections of sections of	potentially	stormwater	-Minimise dust generation where possible by
disturbance of the	hardstand/sealed surface coverings where underlying sub-	containing	system if not	dampening any soils prior to disturbance
surface	surface fill materials are not required to be disturbed	TPHs, BTEX,	appropriately	-Wash hands and clothes after work
coverings/Cap (as	Minor landscaping works on permeable surface coverings	PAHs, metals,	contained.	-A half-face respirator (fitted with organic cartridges) or
defined in Section	installed as part of Cap on sub-surface retained soils:	asbestos		dust-masks must be available for use at the Site in the
3.2)	-Mowing of grassed areas	(though		event that significant odours or dust is generated
	-Raking or placing of additional growing medium on top of	considered		during the works
	existing growing mediums	unlikely to be		-Where materials are required to be imported to the
	-removal of surface weeds or similar	generated		Site for use in reinstatement, repair, refurbishment,
	-shallow planting that does not require contact with the sub-	during Minor		maintenance or similar they must meet the
	surface retained soils (soils greater than 0.2 m depth) or GCL	Works)		requirements set out in Section 4.7 of this EMP.
	in the Northern Remediation Area OR exposure, contact with			-Where off-site disposal is required ensure all
	or other disturbance to the Marker Layer (described in			excavated materials (soils and/or waters) are handled,
	Section 2.3.3);			managed and disposed off-site in accordance with
	Landscaping works in planter beds installed on			NSW EPA (2014) Waste Classification Guidelines or
	impermeable surface coverings (planter beds with			equivalent and disposal to a facility licensed by NSW
	concrete base – no contact with underlying retained soils)			EPA to receive the class of waste material. Waste
	(locations described in Section 3.2)			classification works must be completed by an
	Internal or External Building façade works:			appropriately qualified contaminated site consultant or
	-Painting, pointing, plastering			equivalent. Records of waste classification and disposal
	-Internal fitouts aboveground only			must be maintained and recorded in a materials
	Does not include:			tracking register.
	- planting or any similar landscaping activity that requires			
	excavation into the growing medium or sub-surface at			
	depths beyond the base of the Cap, as set out above			
	-maintenance of the groundwater/seepage collection system			
	-maintenance of underground utilities			



Works	Examples of Types of Works	Possible Risks to Users	Possible Risks to the	Control Measures Required
			Environment	Repairs/Maintenance to Surface Coverings/Cap - Where repairs/maintenance to the surface coverings/Cap are required that are considered to be Minor Works, the Harbour Trust must determine whether an exclusion zone is required to be established around the area subject to the repair or maintenance to preclude further compromise of the surface coverings and/or exposure to the sub-surface of the area by users until such time that repairs/maintenance can be completed. Where an exclusion zone is required then: -The diameter of the exclusion zone is to be determined by those undertaking the works and
				establishing the exclusions zone as part of those works (Site manager, contractors) and must be sufficient to prevent access by normal users of the Site -The exclusion zone is to be established by using physical barriers such as bollards, cones, tape or mesh. Timeframes in which the repairs are required to be completed are provided in Section 3.1 of this EMP.
MAJOR WORKS				
Major works	Landscaping, Maintenance or Other Works within	Inhalation of	Medium risk of	Harbour Trust Approval
comprise activities	Northern Remediation Containment Area:	vapours or	runoff. May	-Any works to be conducted that are considered to be
that require	-where direct contact with the sub-surface soils and/or	dusts	impact	Major Works and that may compromise the integrity of
disturbance to the	groundwater at depths greater than 0.2 m is possible;	potentially	stormwater	the Cap or Cut-off Wall must be approved by the
surface	-where direct contact with the GCL is possible;	containing	system if not	Harbour Trust.
coverings/Cap	-any works that directly affect or disturb the Cut-off Wall;	TPHs, BTEX,	appropriately	-Harbour Trust must not approve such works until they
such that	Any works during which the impermeable surface	PAHs,	contained.	have considered (based on the location and scale of the
underlying	coverings are disturbed such that	asbestos.		Major Works and acting reasonably) whether
materials are	-direct contact with the underlying retained contaminated fill	Dermal		consultation with an appropriately qualified
disturbed or could	material or groundwater is possible or will occur	exposure to		contaminated site consultant is required to obtain
be disturbed and	Construction and maintenance of sub-surface services:	soils and/or		formal advice on whether the works will affect on the
include any works	- gas, electricity, stormwater, surface drainage, telephone,	water		Cap or Cut-off Wall and whether:
associated with the	cabling and water supply;	potentially		1) a Construction Environmental Management Plan
groundwater/		contain TPHs.		(CEMP) is required for the works: and





Works	Examples of Types of Works	Possible Risks to Users	Possible Risks to the Environment	Control Measures Required
				a P3 dust mask/respirator be available during manual handling of soils and/or waters -Wash hands and clothes after work
				Odours, Staining, Sheens or Unexpected Finds -Should odours, sheens or similar or unexpected materials be uncovered works must cease and the Harbour Trust must be immediately notified. The Harbour Trust will determine whether consultation with an appropriately qualified contaminated site consultant is required to determine control measures, which are likely to require the following: - that a Photoionization Detector (PID) with a lamp voltage of at least 10.6 electron volt (eV) be used to monitor operator/ field staff breathing zone for volatile organic contaminants (VOCs) during excavation works conducted where disturbance of the soils will occur; - that Half-face respirators fitted with organic filters and coveralls/Tyvek (chemical resistant) be made available for use and/or are used for future works and/or where further significant contamination /odours are encountered. If deemed required Harbour Trust and/or contaminated site consultant will apply the action levels and personal protective equipment (PPE) requirements for VOCs as presented in Table 4 below for the breathing zone of workers during the works
				General Measures -Prevent dust by applying dust suppression prior to and during excavation; -Any materials excavated must be handled with care; -If soil materials are required to be excavated, excavate and stockpile any hardstand materials, sub-grade



Works	Examples of Types of Works	Possible Risks to Users	Possible Risks to the Environment	Control Measures Required
				materials, topsoils or similar materials separately to the contaminated/potentially contaminated fill materials, cover excavated/stockpiled materials with heavy density polyurethane (HDPE) plastic or similar (appropriate to prevent runoff from materials, water infiltration and prevent dust generation) then use for reinstatement in same location or placement elsewhere on the Site, as determined by the Harbour Trust; - Excavated soils or waters containing or suspected of containing contamination must not be left unattended. If it is necessary to leave materials unattended, the soils are required to be either placed back in the excavation and the surface cover reinstated so that exposure to these materials cannot be gained by users or must be formed into stockpiles that are covered with heavy density polyurethane (HDPE) plastic or similar; waters are required to be stored in appropriate containers that can be sealed and secured; -Where off-site disposal is required ensure all excavated materials (soils and/or waters) are handled, managed and disposal to a facility licensed by NSW EPA to receive the class of waste material. Waste classification works must be completed by an appropriately qualified contaminated site consultant or equivalent. Records of waste classification and disposal must be maintained and recorded in a materials tracking registerWhere materials are required to be imported to the Site for use in reinstatement, repair, refurbishment, maintenance or similar they must meet the requirements set out in Section 4.7 of this EMP.



Works	Examples of Types of Works	Possible Risks to Users	Possible Risks to the Environment	Control Measures Required
				Repair/Maintenance of Surface Coverings/Cap
				been compromised such that the required
				repair/maintenance works are considered to be Major
				Work or any works are undertaken that are Major
				Works, an exclusion zone must be established around
				the impacted area to preclude exposure to the sub-
				surface by users until such time that repairs or the
				works can be completed.
				-The diameter of the exclusion zone is to be
				determined by those undertaking the works and
				establishing the exclusions zone as part of those works
				(Site manager, contractors) and must be sufficient to
				prevent access by normal users of the Site;
				-The exclusion zone is to be established by
				using physical barriers such as secure lockable fencing
				or similar. Timeframes in which the
				repairs/maintenance works are required to be
				completed are provided in Section 3.1of this EMP



Table 4: Breathing Zone Action Levels

Chamical Hazarda	Risk: H - High	TWA (mg/m ³)	STEL (mg/m ³)	IE (eV)	MUL Act (75% saf	LEL (%)			
Chemical Hazarus.	M - Med L - Low				No APR	½ face APR	full face APR	IDLH reached	
Benzene	Н	16	-	-	5	37.5	187.5	3000	1.3-
** Remember - Max Use Limit (MUL) = (Protection Factor x Exposure Limit) x 75% ** PF= 10 for ½ face, 50 for full face, 10000 for SCBA									

4.4 Control Measures - Asbestos

Should fragments of asbestos containing materials (ACM) and/or bonded fibrous cement materials (potential ACM) and/or any quantity of friable material be encountered the following procedure is required to be implemented:

- Works must cease and the exposed area should be covered with substantial plastic sheeting that is securely anchored to the ground surface and be enclosed within a barrier to prevent access.
- The Site Manager must be immediately notified;
- The Site Manager must determine if appropriate signage should be displayed to warn of the presence of these materials;
- A suitably qualified Occupational Hygienist or equivalent must be contacted by the Site Manager to provide an assessment of risks and the required management response and control measures;
- No further works are to be undertaken on the Site until the Site Manager has provided approval for works to re-commence.
- Asbestos removal works must only be undertaken in accordance with the requirements of the relevant OH&S regulations and NSW Workcover;
- A bonded asbestos licence is required to be issued by NSW Workcover (or as superseded at the time of works) to remove, repair or disturb more than 10 square metres of bonded asbestos material such as fibro, corrugated cement sheeting and asbestos cement pipes. A friable asbestos licence is required to be issued by NSW Workcover to remove, repair or disturb any amount of friable asbestos, such as sprayed limpet, asbestos cloth, millboard and pipe lagging. This licence also allows the removal of bonded asbestos;
- NSW WorkCover must be notified seven days before removing bonded asbestos and a work site permit from NSW WorkCover is required to be obtained before removing any friable asbestos. Applications must be lodged at least seven days before the proposed work is due to start.

Should the procedures above be required to be implemented, records of such works will need to be completed and retained by the Supervisor or person in charge of works. Such records would include the location on the Site where materials were found, reason for the works, the type of asbestos, documentation of any monitoring or clearance works completed by the Occupational Hygienist, report on waste classification works completed by the environmental consultant on any removed materials and the receiving waste facility dockets.

4.5 Handling of Contaminated and Potentially Contaminated Materials

Any solid or liquid materials excavated from the surface and sub-surface of the Site must be handled with care and in accordance with the requirements set out above. Any materials that require disposal off-site will require the engagement of a suitably qualified environmental consultant to conduct appropriate sampling and analysis to



determine the requirements for off-site disposal in accordance with NSW EPA (2014) guidelines or equivalent. Records of any materials excavated and then subsequently disposed off-site are required to be completed and retained by the Site Owner. Such records would include the location on the Site where materials were excavated, reason for excavation works, the type of materials excavated, report on waste classification works completed by the environmental consultant on the excavated materials and the receiving waste facility dockets.

4.6 Unexpected Finds

In the event of unexpected finds e.g. presence of an underground storage tank, drums, pipework or similar, works must cease and the Site Manager and/or Site Owner must inspect the unexpected find and isolate the area until such time that an appropriately qualified environmental consultant conducts an inspection and provides a recommendation of what works are required to control/manage the unexpected find in accordance with the relevant guidelines made or approved by the NSW EPA, NSW regulations and legislation.

Records of any unexpected finds are required to be completed and retained by the Site Owner. Such records would include the location on the Site where the unexpected find was uncovered, how it was found, the follow up works undertaken, any reports, advice or similar provided by the environmental consultant, documentation, reports or similar on any rectification/remediation works undertaken and photographic record.

4.7 Imported Materials

Materials to be imported on the Site to be used as part of any refurbishment, rectification, repair or maintenance work either as part of the Cap or in areas beneath the Cap must satisfy the following criteria:

- Virgin excavated natural materials (VENM) must satisfy the criteria stated in NSW EPA (2014) and NSW EPA (2017) guidelines and be demonstrated to be:
 - Natural material (such as clay, gravel, sand, soil or rock fines);
 - Materials that has been excavated or quarried from areas that are not contaminated with manufactured chemicals or process residues, as a result of industrial, commercial, mining or agricultural activities; and
 - o Materials that do not contain any sulfidic ores or soils or any other waste;
- Topsoils, growing media, mulch etc. for landscaping purposes must not contain foreign substances, suspicious staining and/or odours;
- Materials that are permitted to be brought onto the Site via an exemption issued under the POEO Act must meet and comply with the requirements of any such exemption, prior to, during and/or after importation of materials;
- Any materials proposed to be imported to the Site must not contain any of the following:
 - o Marine mud, peat, soluble or perishable materials,
 - o Dangerous or toxic material or material susceptible to combustion;
 - o Metal, rubber, plastic or synthetic material or other forms of general rubbish;
 - o Construction/demolition debris; and/or
 - o Asbestos.



4.8 Reporting of Complaints and Incidents

If a complaint is made by a member of the public or by any other person with respect to any environmental management or control issue either during "Minor Works" or during "Major Works" or at any other time, appropriate corrective action is required to be undertaken as soon as practicable. The Site Owner is responsible for ensuring the corrective action is undertaken (as set out in Table 1).

Similarly, if an environmental incident occurs that has given or may give rise to pollution of soil, air or waters, appropriate corrective action is required to be undertaken as soon as practicable.

In addition to the above, complaints and environmental incidents are required to be notified to the Site Owner as soon as practicable after a complaint has been made or an environmental incident has occurred. If appropriate, and following the Site Owner's instructions, notification may need to be made to the applicable regulatory authority.

Records of complaints and incidents are required to be entered into a register to be developed for the Site, but only after corrective action has been taken and the Site Owner has been notified.



5 Limitations

This Long-term Environmental Management Plan has been prepared for the sole purpose of documenting the procedures that are required to be implemented on the Site in accordance with generally accepted consulting practice. No other warranty or guarantee, expressed or implied is made as to the advice indicated in this report.

This report should not be used for any other purpose without our prior written consent. Accordingly, neither CONSARA nor any member or employee of CONSARA accepts responsibility or liability in any way whatsoever for the use of this report for any purpose other than that for which it has been prepared.

This report should not be released to any other party, in whole or in part, without the express written consent of CONSARA. CONSARA accepts no liability or responsibility whatsoever for or in respect of any use or reliance upon this report by any third party.

CONSARA has relied upon and presumed accurate information provided by Sydney Harbour Federation Trust and/or any third party (or absence thereof) in making the assumptions made in this report. Nothing in this report should be taken to imply that CONSARA has verified or audited any of the information supplied to us other than as expressly stated in this report. We have assumed this information to be both adequate and accurate for the purposes of this report.

Where findings, observations and conclusions are based solely upon information provided by Sydney Harbour Federation Trust and/or a third party and CONSARA do not accept, to the maximum extent permitted by law, any liability for any losses, claims, costs, expenses, damages (whether in statute, in contract or tort for negligence or otherwise) suffered or incurred by Sydney Harbour Federation Trust or any third party as a result of or in connection with CONSARA's reliance on any such the information to the extent that such information is false, misleading or incomplete and CONSARA give no warranty or guarantee, express or implied as to such findings, observations and conclusions.

If further information becomes available, or additional assumptions need to be made, CONSARA reserves its right to amend any statements or opinions made in this report.



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Figures

Figure 1: Site Location Plan

Figure 2: Site Layout Plan


SOURCE: NEARMAP - IMAGE DATED 7th APRIL 2024



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Appendix A: Figures from Validation Report











	description	drawn	approved	date	IMAGE SOURCE: maps.six.nsw.gov.au/	drawn	MV		cli
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Appendix B: Registered Survey Plan of Final Surfaces of Northern Remediation Area



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DWG STATUS	SHEET SIZE			
DRAFTSMAN	PROJECT	MANAGER	SCALE	A1
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9852		(CO1	G

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ESM3	EXISTING SEALED	ε	2.89	3.11	+0.2
ESM4	EXISTING SEALED	ε	2.85	3.10	÷0.2
ESM5	EXISTING SEALED	ε	2.87	2.97	+0,1
ALL PIT FOR EX	TS ARE TO FINISH FLUSH ISTING PITS WITH NON- NDARD SIZED PITS ON I	I WITH NEN STANDARI DRAWING N	W PAVEMENT D DIMENSIONS No 9852-C02	, REFER DETA	ILS FOR

BE CONSTRUCTED OVER EXISTING FOR DETAILS REFER TO SECTIONS

ESP7

ESP13

EP1

EP7

EXISTING SEALED

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EP5 EXISTING SEALED

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ESM1 EXISTING SEALED

THEISS SERVICES PTY LTD

-NOTE: NEW SUBSOIL DRAINAGE TO CONNECT INTO EXISTING DRAINAGE LINES. FOR LOCATION OF EXISTING DRAINAGE LINES REFER TO STAGE 2 REMEDIATION WORK-AS-EXECUTED DOCUMENTATION PREPARED BY

SL 3.64 INV 3.49

-NEW GRATED DRAIN

/(RL9.85)

NOTES

THE POSITION OF ALL EXISTING SERVICES SHOWN SHOULD BE REGARDED AS APPROXIMATE ONLY AND NOT NECESSARILY COMPREHENSIVE. IT IS THE CONTRACTORS RESPONSIBILITY TO DETERMINE THE EXACT LOCATIONS AND INFORM ALL AUTHORITIES PRIOR TO ANY EXCAVATION. FOR SETOUT OF NEW STRUCTURES

REFER TO LANDSCAPE ARCHITECTS DRAWINGS

	LEGEND
(RL 2.62)	NEW DESIGN SPOT LEVEL
· <u>`</u>	NEW Ø150 (UNO) UPVC STORMWATER DRAINAGE PIPE (SIZE AS SHOWN ON PLAN) LAID AT 1.0% MIN. FALL TO OUTLET
SP#	NEW CONCRETE STORMWATER PIT, REFER TO TYPICAL PIT DETAIL ON DRAWING No 9852-C02, REFER TO PIT SCHEDULE FOR SURFACE TREATMENT DETAILS
GD#	GRATED DRAIN, MIN 0.5% FALL TO OUTLET, REFER TO SECTIONS ON DRAWING 9852-C02
$ \rightarrow $	OVERLAND FLOW PATH
FPo	NEW FLUSHING POINT FOR DETAILS REFER TO DRAWING No 9852-C02
- 55	NEW Ø100 SUBSOIL DRAINAGE PIPE C/W FILTER SOCK, LAID AT MIN. 1.0% FALL TO OUTLET
¢450 — SW —	EXISTING STORMWATER DRAINAGE PIPE & SIZE
🔀 ESP#	EXISTING STORMWATER PIT, REFER TO PIT SCHEDULE FOR SURFACE TREATMENT DETAILS
EP#	EXISTING PIT (NON-STORMWATER), REFER TO PIT SCHEDULE FOR SURFACE TREATMENT DETAILS
⊗ esm#	EXISTING SEWER MANHOLE, REFER TO PIT SCHEDULE FOR SURFACE TREATMENT DETAILS
	NEW DETAINING WALL

NEW RETAINING WALL REFER TO DETAILS ON DRAWING 9852-C06

EXISTING RETAINING WALL ----- BOUNDARY LINE PIT SURFACE TREATMENT SCHEDULE NOTE: WHERE EXISTING PITS ARE TO BE RAISED/LOWERED, REFER TO TYPICAL PIT DETAILS ON 9852-C02 PIT SURFACE LEVELS (m) PIT COVER TYPE DUT NÖ EXTG SL | NEW SL | DIFFERENCE SP1 NEW GALV. GRATED B N/A 4.83 N/A 2.98 SP2 NEW GALV. GRATED B N/A N/A SP3 NEW GALV. GRATED B N/A 3.54 N/A ESP1 f 0

3.20 3.20 EXISTING TBC ESP2 EXISTING TBC 3.25 3.20 ESP3 EXISTING GRATED 3.05 3.02 ESP4 EXISTING GRATED 3.00 3.16 3.03 3.10 ESP5 EXISTING SEALED

-0.05 -0.03 -0.16+0.07 ESP6 EXISTING GRATED 2.90 -0.103.00 EXISTING GRATED 2.97 2.93 -0.04 ESP8 EXISTING SEALED 3.07 2.95 -0.12 3.05 ESP9 EXISTING SEALED 3.07 -0.02 ESP10 EXISTING SEALED 2.88 3.09 +0.21 2.95 ESP11 EXISTING GRATED 2.99 -0.04ESP12 EXISTING GRATED 2.90 2.93 +0.03

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Appendix C: Registered Survey Plan of Cap Thickness across Northern Remediation Containment Area



	DWG STATUS	SHEET SIZE				
PARK	DRAFTSMAN PROJECT MANAG		MANAGER	SCALE	A1	
	AC	N	1M	A1 - 1:150		
I	PROJECT REF No.		DRAWING No.		REVISION	
	9852		CS	SK10	A	



Appendix D: Cut-Off Wall Location and As-Builts



G	29.7.13	UPDATED FOR B	_DG 11 WORKS	BG	DAC	SHB/MC	
F	30.4.2013	FIX ERROR IN LE	GEND	BG	DAC	SHB/NS	
Е	5.4.2013	UPDATED FOR SI	EAWALL INSTALLATION INFORMAT	BG	DAC	SHB/NS	
D	7.2.2013	AS-BUILT NORTH	H & SOUTH WING WALLS	RK	DC	SHB/NS	
С	5.2.2013	FOR REVIEW	FOR REVIEW				SHB/NS
В	8.1.2013	AS-BUILT SHEET	AS-BUILT SHEET PILE WALL, COL LOCATIONS				SHB/NS
А	28.11.2012	ISSUED FOR REVIEW				DC	SHB/NS
REV	DATE		DESCRIPTION	BY	APP'D	REVIEWED	
			2		3		1







Appendix E: Registered Survey of Extent of GCL and As-Builts



13	14	15	16	
				к
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	Building	11		F
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SERVICES PTY LTD 10 725 247 9 010 725 247 JP STREET, ATTA, NSW 2150 02) 8892 5901 2) 8892 5901	HMAS PLATYPUS DRAWING TITLE BULK EARTHWOR AS BUILT	STAGE 2 REMEDIA	ATION WORKS	
12	A1 1:150	050010-CAP-WA	AE-012 A	





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Appendix F: Specifications for Liners used in Cap

bidim[®] Nonwoven Geotextiles

Technical Data Sheet



Specifications

bidim[®] Geotextiles – Typical and MARV Values

bidim® geotextiles are manufactured in accordance to ISO 9001:2008, Cert No: QEC1773.

All bidim® "A" range nonwoven, needle punched geotextiles are made in Australia.

	Test	Standard	Unit	S	A12	A14	A19	A24	A29	A34	A39	A44	A49	A64
Mechanical Properties	Wide Strip Tensile Strength (MD/XMD)	AS3706.2-12	kN/m	Typical MARV	9.4/8.0 7.5/6.6	11.0/9.5 9.0/7.7	14.0/12.8 12.0/10.0	16.0/14.2 14.0/12.0	19.0/17.0 15.5/14.5	21.5/21.0 18.5/17.5	26.5/25.5 22.0/21.0	29.5/28.0 24.5/24.0	37.0/34.0 31.5/30.5	41.5/38.0 35.8/33.0
	Grab Tensile Strength (MD/XMD)	AS3706.2-12	N	Typical MARV	620/570 510/430	720/650 600/530	950/860 795/730	1,130/1,060 850/800	1,280/1,200 1,100/1,000	1,430/1,350 1,270/1,210	1,900/1,670 1,590/1,490	2,100/1,910 1,800/1,680	2,850/2,570 2,490/2,200	3,010/2,850 2,620/2,460
	Trapezoidal Tear Strength (MD/XMD)	AS3706.3-12	N	Typical MARV	240/230 205/180	300/270 240/220	350/330 295/280	430/400 320/310	490/450 380/360	540/510 440/410	630/610 530/510	753/700 575/550	915/910 750/740	1,060/1,010 800/770
	CBR Burst Strength	AS3706.4-12	N	Typical MARV	1,550 1,275	1,720 1,500	2,250 1,925	2,700 2,450	3,200 2,800	3,600 3,300	4,400 3,950	4,800 4,450	6,400 5,850	6,850 6,300
	G Rating	Austroads	G	Typical MARV	1,150 900	1,500 1,250	1,950 1,550	2,230 1,800	2,480 2,160	2,800 2,510	3,450 3,100	3,900 3,450	5,150 4,600	5,400 5,000
(MD) = (XMD) =	(MD) = Machine Direction Strength (XMD) = Cross Machine Direction Strength													
ties	Pore Size	AS3706.7-03	μm	Typical	120	110	110	100	90	90	80	80	80	80
roper	Permittivity	AS3706.9-12	S ⁻¹	Typical	2.50	2.35	2.20	2.00	1.85	1.65	1.25	1.10	0.90	0.80
raulic	Coefficient of Permeability	AS3706.9-12	m/s x 10-4	Typical	33	33	33	33	33	33	33	33	33	33
Hydı	Flow Rate @ 100mm Head	AS3706.9-12	I/m²/s	Typical	250	235	220	200	185	165	125	110	90	80

The data and specifications contained in this table are obtained from the manufacturer's laboratory testing. To ensure this information is current please contact your local branch of Geofabrics Australasia.

The product properties listed on this sheet include both Typical and Minimum Average Roll Values (MARV) for machine and cross machine directions (MD/XMD).

Definitions of these terms are included on the reverse side of this data sheet. All testing has been carried out by a NATA accredited laboratory and copies of test certificates are available on request.



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Definition of Terms

ISO Accreditation	ISO9001 is a manufacturing quality assurance system under which bidim [®] is manufactured. Please refer to the bidim [®] Quality Assurance & Control Manual for testing frequencies. Note; not all manufacturers test to the same frequency.
Machine Direction (MD)	The direction in a machine-made fabric, parallel to the direction of motion of the material through the processing machine (i.e. along the length of the roll).
Cross Machine Direction (XMD)	The direction in a machine made fabric, perpendicular to the direction of motion of the material through the processing machine (i.e. across the width of the roll).
Typical Value	A typical value is the arithmetic mean of a set of results (refer to diagram below). This implies that 50% of the tested specimens will typically exceed this value and 50% will typically not meet this value.
Minimum Average Roll Value (MARV)	MARV is a statistical derivation for any distribution of data. It is defined as the mean or typical value less 2 standard deviations (refer to diagram below). Mathematically it is implied that 97.5% of the tested specimens will exceed the MARV.

Indicative Results Spread

(for a given test method for a given period of time)



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ELCOSEAL® Geosynthetic Clay Liners

Technical Data Sheet





QUALITY - SUPPORT - EXPERTISE

ELCOSEAL[®] Bentonite Specification

ELCOSEAL[®] Geosynthetic Clay Liners contain a sodium bentonite processed to exceed the relevent requirements of API Spec 13A, for which the bentonite supplier is a licensed manufacturer. The unique swelling properties and low permeability performance of ELCOSEAL[®] 's sodium bentonite can be attributed to the following properties.

PROPERTY	TEST METHOD	UNITS	VALUE
Dontonito Dortialo Cino	Dry Screen	% passing 75 μ m	≥ 75
Dentonite Particle Size	AS 1289-3.6.2 ¹	% ≤ 0.5µm	≥ 55
Swell Index	ASTM D 5890 ²	ASTM D 5890 ² mL/2g	
Fluid Loss	ASTM D 5891 ³	D 5891 ³ mL	
Montmorillonite Content	XRD Quantitative Mineralogy Analysis ⁴	% of Bulk Sample	≥ 70
Montmorillonite Content of Bentonite Particles $\leq 0.5 \mu m$	XRD Quantitative Mineralogy Analysis ⁴	% ≤ 0.5µm	≥ 90
Calcium Carbonate Content (CaCO ₃)	XRD Quantitative Mineralogy Analysis ⁵	% of Bulk Sample	≤2
Layer Charge and Layer	Chemical analysis and structural formula	e⁻ per unit cell (O ₂₀ (OH)₄)	0.75 - 0.90
Charge Distribution	calculation ⁶	% in tetrahedral sheet	< 35
Cation Exchange Capacity (CEC)	NH4 displacement ⁷ Barium Saturation Method - (e.g. Battaglia et al., 2006) ⁷	cmol/kg of Bulk Sample	70 - 110
	Methylene Blue ⁷		

EXPLANATION OF TEST METHODS

- BENTONITE PARTICLE SIZE AS 1289-3.6.2 'Methods of testing soils for engineering purposes - Soil classification tests - Determination of the particle size distribution of a soil - Analysis by sieving in combination with hydrometer analysis (subsidiary method)' – Particle size provides an indication of the reactive surfaces. Smaller particle sizes generally react more efficiently and effectively with water to result in better swelling, lower fluid loss, higher swelling pressure, greater gel strength and lower permeability.
- 2. SWELL INDEX ASTM D5890 'Standard Test Method for Swell Index of Clay Mineral Component of Geosynthetic Clay Liners' – This is an index test that evaluates the swelling potential of the bentonite component of a GCL. The index relates to bulk swelling under minimal confinement. The swell index is generally inversely related to GCL permeability, ie. the higher the swell index, the lower the GCL permeability.
- 3. FLUID LOSS ASTM D5891 'Standard Test Method for Fluid Loss of Clay Component of Geosynthetic Clay Liners' This is an index test that evaluates the fluid loss properties of the bentonite component of a GCL, under 100psi (690 MPa) pressure, over a specified period of time. A low Fluid Loss value is indicative of the ability of the bentonite to restrict movement of liquid under load. Fluid Loss is directly related to GCL permeability, ie. the lower the fluid loss, the lower the GCL permeability.
- 4. MONTMORILLONITE CONTENT XRD Quantitative Mineralogy Analysis The method enables quantification of the mineralogy of a bentonite by powder X-ray diffraction and Reitveldt refinement. The XRD method can be performed on bulk bentonite, showing montmorillonite content as a % of bulk; or on size-fractionated materials, showing montmorillonite content as a % of size fraction (ie. s0.5µm).

REFERENCE

Taylor, J.C., Hinczak, (2004). Reitveldt Made Easy. Sietronics, Pty Ltd, Belconnen Australia, 201p.

- CALCIUM CARBONATE CONTENT 'XRD Quantitative Mineralogy Analysis' As for Montmorillonite content, quantitative mineralogy analysis enables quantification of carbonate minerals present in bentonite.
- 6. LAYER CHARGE AND LAYER CHARGE DISTRIBUTION 'Chemical analysis and structural formula calculation' This method enables calculation, from chemical analysis, of layer charge characteristics of the <0.2 micron smectite clay component of a GCL. Chemical analysis is quantified by XRF on calcium saturated purified smectite samples. Smectite layer charge is responsible for the cation exchange capacity of bentonite, but also influences swelling, sealing and gel formation. Layer charge values <0.90e- per unit cell in combination with a tetrahedral layer charge <35%, indicates a good swelling bentonite.</p>

REFERENCES

 Norrish, K., Hutton, J.T. (1969). "An accurate X-ray spectroscopic method for the analysis of a wide range of geologic samples", Geochemical Cosmochimical Acta, 33, 431-453.

2.Bodine, M.W.Jr. (1987). "CLAYFORM: A FORTRAN 77 computer program apportioning the constituents in the chemical analysis of a clay or other silicate mineral in a structural formula", Computers and Geosciences, 13:77-88.

7. CATION EXCHANGE CAPACITY – 'NH₄ Displacement Method' – 'This method has traditionally been used to determine CEC in SOILS. The NH₄ displacement method can be modified to enable direct determination of the cation exchange capacity of the bentonite component of a GCL. The amount of NH₄ retained by the clay is quantified by Inductively Coupled Plasma Atomic Absorption Spectrometry (ICP-AAS). This method can be performed on the bulk bentonite, in which case "Bentonite CEC" is recorded; or on the monomineralic smectite (<2.0 micron) isolated from the bentonite, in which case the "Smectite CEC" is recorded. The ranges in CEC values specified provide the best combination of good swelling and cation retention capabilities – see Note 6 - LAYER CHARGE AND LAYER CHARGE DISTRIBUTION.

or 'Barium Saturation Method' – 'The barium (Ba) displacement method enables direct determination of the cation exchange capacity of the carbonate free fractions of the bentonite component of a GCL. The amount of Ba retained by the clay is quantified by X-ray Fluorescence (XFF). This method can be performed on the carbonate free bulk bentonite, in which case "Bentonite CEC" is recorded; or on the carbonate free monomineralic smectite isolated from the bentonite, in which case the "Smectite CEC" is recorded. The ranges in CEC values specified provide the best combination of good swelling and cation retention capabilities – see Note 6 – LAYER CHARGE AND LAYER CHARGE DISTRIBUTION.

REFERENCES

Wang, M.K., Wang, S.L., Wang W.M. 1996. Rapid Estimation of Cation-exchange Capacities of Soils and Clays with Methylene Blue Exchange. Soil Science Society of America J. 60:138-141.

ELCOSEAL[®] GCL Specification

ELCOSEAL[®] is a New Generation Geosynthetic Clay Liner (GCL) made from quality polypropylene geotextiles and premium grade sodium bentonite powder mined in Australia. ELCOSEAL[®] GCL's are fibre-reinforced by needle-punching the composite across the entire surface area of the product. Unique to this product, the high tenacity fibres are then thermally-locked to ensure high long-term shear strength.

οράρερτγ		TEST	MQC ¹		ELCOSEAL® GRADE				
PROPERTY		METHOD	FREQUENCY	UNITS	X800	X1000	X2000	X3000	
GCL Hydraulic Properties									
Hudroulia Conductivity k	MaxArv ²		40.000m ²	m/a	3.5 x 10 ⁻¹¹	2.8 x 10 ⁻¹¹	3 x 10 ⁻¹¹	2 x 10 ⁻¹¹	
Hydraulic Conductivity, K	Typical ³	ASTIVI D 5007	40,0001112	III/S	2 x 10 ⁻¹¹	1.6 x 10 ⁻¹¹	1.6 x 10 ⁻¹¹	1.5 x 10 ⁻¹¹	
GCL Components - Mass									
Cover Nonwoven Geotextile Mass	MARV ⁴	AC 2706 1	10.000m ²	a/m2	220	270	270	290	
per Unit Area	Typical	AS 3700.1	10,000111-	g/III-	280	300	300	330	
Bentonite Mass per Unit Area @	MARV	ASTM D 5993	2 500m ²	a/m²	3,600	4,000	3,700	4,250	
0% moisture content	Typical	A21M D 2992	2,300115	g/III-	4,100	4,500	4,250	4,700	
Carrier / Composite Geotextile	MARV	AC 070C 1	70.000m²	a /m2	110	110	380	380	
Mass per Unit Area	Typical	AS 3700.1	70,000111-	g/m²	110	110	410	410	
Geotextile Configuration (Carrier / C	over)				W / NW ⁵	W / NW	W+NW/NW	W+NW/NW	
GCL - Mass									
GCL Total Mass per Unit Area @	MARV	ASTM D 5993	2,500m² g/	a/m²	3,930	4,380	4,350	4,920	
0% moisture content	Typical	ASTINI D 5993		g/m-	4,490	4,910	4,960	5,440	
GCL - Strength Properties									
Strip Tensile Strength (MD) ⁶	MARV		10.000m ² kN/m	kN/m	7	8	12	12	
	Typical	ASTIVI D 0700	10,000111-	KIN/III	10	11	15	16	
CDD Strongth	MARV	AC 2706 4	05.000m²	25.000m ² N	1,400	1,600	3,900	4,100	
CDR Suengui	Typical	AS 3700.4	23,0001112	IN	2,000	2,100	4,900	5,300	
CDD Elemention	MARV	AC 2706 4	25,000m ² %	0/	10	15	30	30	
CDR Elongation	Typical	AS 3700.4		%	30	40	80	80	
GCL - Shear Strength Properties									
Hydrated Peak Internal Shear Strength @ 10kPa Normal Stress	Typical ⁷	ASTM D 6243	Periodic	kPa	30	30	35	40	
Hydrated Peak Internal Shear Strength @ 30kPa Normal Stress	Typical	ASTM D 6243	Periodic	kPa	50	50	60	70	
GCL Longitudinal Edge Treatment									
Bentonite Impregnation - Width ≥ 300mm - Typical					\checkmark	\checkmark	\checkmark	\checkmark	
Edge Sealing					\checkmark	\checkmark	\checkmark	\checkmark	
GCL Roll Dimensions									
Roll Dimensions (Width x Length)				m	4.7 x 45	4.7 x 35	4.7 x 30	4.7 x 30	
Maximum Roll Mass (Weighed every roll)					1,200	1,000	875	950	
GCL Spreader Bar Requirement					Heavy-Duty8	Heavy-Duty8	Standard ⁹	Standard ⁹	

NOTES

- MQC = Manufacturing Quality Control an ongoing system that monitors and tests materials during manufacture to ensure compliance with certification documents and contract specifications.
- MaxARV = Maximum Average Roll Value a MaxARV is defined as the Mean or Typical values plus 2 standard deviations. Mathematically, it is implied that 97.5% of the results of the tested specimens will be less than the MaxARV. A MaxARV provides a confidence level of 97.5%. NOTE – in reference to GCL Permeability, LOWER IS BETTER.
- 3. **Typical** = A typical value is the arithmetic mean of a set of results. This implies that 50% of the tested specimens will typically exceed this value and 50% will typically not meet this value.
- 4. MARV = Minimum Average Roll Value a MARV is defined as the Mean or Typical values less 2 standard deviations. Mathematically, it is implied that 97.5% of the results of the tested specimens will exceed the MARV. A MARV provides a confidence level of 97.5%.
- 5. W= Woven, NW= Nonwoven.
- 6. **MD** = Roll Machine Direction.
- Peak value reported at 10kPa or 30kPa normal stress. [The reported values are not intended to replace site specific internal shear or interface friction testing required for design].
- 8. Heavy-Duty WLL (Working Load Limit) = 1400kg.
- 9. Standard WLL = 1000kg.

ELCOSEAL® Technical Data Sheet

Contact your nearest Geofabrics Australasia office for any further information on:



General Information



GCL Technical Notes



ELCOSEAL[®] Installation Guidelines



Bentonite Technical Notes



Manufacturers Quality Assurance & Control Manual

MODEL GCL SPECIFICATION
Scope - This specification details the technical requirements for the supply and installation of a needle- pandhed Geografishic Caly Liner (CCL). The materiality intrelated and installation performed shall be in strict accordance with these requirements and the corrupt drawing.
The following assocification guideline reflects industry accepted installation procedures and current guality centrol last protocol. It is to be used as the general format, not as a direct substitute for a project specific Geosynthetic Clay Liner (GCL) specification.
DEFINITION OF TERMS
For the purposes of this specification the following definitions shall apply:
CBR - California Bearing Ratio - 50mm Plunger
COA - Construction Quality Assurance - A planned system of activities that provide assurance that the facility was constructed as specified in the design.
Cu - Coefficient of Uniformity - D60 / D10
Geosynthetic Clay Liner (GCL) - A factory manufactured hydraulic barrier consisting of powdered Sodium Bencionia city, sundividued between, supported and encapsulated by two geotextiles, held bagether by needle- sundhirp.
Geotextile - Woven or normoven fabrics used to contain the bentonite in a GCL.
Index Test - This is a product acceptance test used for quality control and classification of various grades of GCL.
MARY – Minimum Average Roll Value – A statistical value which represents the mean loss 2 standard deviations, effectively providing a 97.5% confidence limit. This value is based on either project testing or historical table of a particular property.
MaxARV – Maximum Average Roll Value – A statistical value which represents the mean plus 2 standard doubtions, offsctively providing a 97.5% confidence limit. This value is based on either project testing or historical data of a particular property.
MOA – Manufacturing Quality Assurance – A planned system of activities that provides assurance that the materials were constructed as specified in the certification documents and contract specifications.
Needle-punching - A GCL manufacturing process whereby boards of barbed needles incorporate the staple fibres from a nonnover gootexile, through a Sodium Benfortile (lay layer, into the matrix of a second or more gootexile bayes.
Performance Test - A test on a particular product or process typically used to simulate actual field conditions.
PP - Polypropylene polymer
Thermal Locking - A needle-punching enhancement process utilising heat to bind the needle-punched libres between the upper and lower geotextile layers to increase the internal shear strength characteristics of GCL's

GCL Model Specification

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The information contained in this brochure is general in nature. In particular the content of this brochure does not take account of specific conditions that may be present at your site. Site conditions may alter the performance and longevity of the product and in extreme cases may make the product wholly unsuitable. Actual dimensions and performance may vary. If your project requires accuracy to a certain specified tolerance level you must advise us before ordering the product from us. We can then advise whether the product will meet the required tolerances. Where provided, installation instructions cover installation of product in site conditions that are conducive to its use and optimum performance. If you have any doubts as to the installation instructions or their application to your site, please contact us for clarification before commencing installation. This brochure should not be used for construction purposes and in all cases we recommend that advice be obtained from a suitably qualified consulting engineer or industry specialist before proceeding with installation. © Copyright held by Geofabrics Australasia Pty Ltd. All rights are reserved and no part of this publication may be copied without prior permission.





INTERMAS NETS,S.A. Ronda de Collsabadell, 11 E-08450 Llinars del Vallès (Barcelona) SPAIN TEL. 34-93-842 57 00 FAX 34-93-842 57 01 web: www.intermas.com e-mail: geo@intermas.com

INTERDRAIN

Drainage geonet

M 5



PRODUCT DESCRIPTION:

INTERDRAIN M5 is a high-density polyethylene (HDPE) geonet. The geonet is made with 2 overcrossed strands at 60°, whose geometry create channels with a high flow capacity, also under pressure and at very low gradients.

FUNCTION:

DRAINAGE.

MAIN USES:

Landfill capping, new landfills, water reservoirs, horizontal drainage in embankments and platforms of roads, railways, trams and other trafficked areas, retaining structures, bridges, foundations, basements, canals, cut and cover tunnels, tunnels and other underground structures, gardens and sport fields.

Characteri	stics	Value	Unit	Standard
Drainage g	jeonet			
Polymer	-	High-density polyethyle	ene(HDPE)	
Carbon	black	1,2 - 2,5	%	ASTM D 1603
Density		> 0,94	g / cm ³	ASTM D1505
Thickne	ss at 2 kPa / 200 kPa	5,2 / 4,8	mm	ISO 9863-1
Mass pe	er unit area	650	g / m²	ISO 9864
Peak ter	nsile strength MD / CD	7,0 / 2,5	kN / m	ISO 10319
Elongati	on at peak, MD / CD	20 / 90	%	ISO 10319
CBR		0,6	kN	ISO 12236
Flow ca	pacity in their plane, MD		l/m·s	ISO 12958 ⁽¹⁾
	σ = 20 kPa	2,00		
i - 1 0	σ = 50 kPa	1,82		
1 – 1,0	σ = 200 kPa	1,44		
	σ = 400 kPa	1,00		

INTERDRAIN standard roll format is 2 or 4 meters-wide.

 $^{(1)}$ ISO 12958-1999 with 380*300 mm specimens and rigid plates (hard-hard). The tolerance is $\pm 30\%$.

i : Hydraulic gradient

MD : Machine direction (longitudinal)

CD : Cross machine direction (transversal)

σ: Normal stress



This information are typical values based on our present state of knowledge and is intended to provide general notes on our products and their uses.



General Brochure





QUALITY - SUPPORT - EXPERTISE
Please read the important notice at the end of this brochure

Megaflo[®] panel drain provides the dimensional stability and field-proven structural strength for quick, effective subsurface drainage. **Megaflo**[®] consists of a perforated HDPE core wrapped with **bidim**[®] nonwoven geotextile to prevent soil ingress into the drainage system.

Performance is the distinguishing feature of the panel drain due to its ability to rapidly collect and remove water. Compared to 100mm diameter round pipe, **Megaflo**[®] has twice the inflow capacity for an equivalent length and will drain water in less than 60% of the response time. Its slim 40mm wide profile permits faster and more cost effective installation in a narrower trench.

The design of the **Megaflo**[®] panel drain permits significantly higher flow velocity at lower head.

FUNCTIONS



Drainage

Removal of excess water from a structure whether it be a road, retaining wall or sports field is key to long term performance of the structure. **Megaflo**[®] is a unique composite drainage system with an unsurpassed infiltration rate and rigid flow path which ensures effective drainage under the most challenging conditions.





Rapid Response Time

Rapid removal of excess water is a key consideration in roads and sports fields where excess water can result in significant damage to the pavement or limit the playability of the sports surface. The long flat shape of **Megaflo**[®] incorporates a high open area for inflow of water - allowing rapid response time well in excess of conventional drainage systems.



Strength



Vertically installing the **Megaflo**[®] drainage panel utilises the ribbed structure to provide higher strength under traffic loads. The properties of high compressive modulus, longitudinal stiffness and structural rigidity aids the mechanical performance of **Megaflo**[®], ensuring the long term hydraulic flow capacity of the drain. High structural strength allows for a minimum cover depth of 100mm, recommended for **Megaflo**[®] in most applications. **Megaflo**[®] typically requires less backfill in comparison to a traditional trench which results in cost savings and faster installation.



APPLICATIONS

ROADSIDE EDGE DRAINS

Megaflo[®] provides faster and higher inflow capacity due to its high trench installation profile and earlier interception of pavement infiltration. **Megaflo**[®] has a high compressive modulus and structural rigidity (preventing deflection under normal service loads), due to its elongated ribbed profile incorporating internal support.

Simple installation techniques provide installation savings compared to a traditional round pipe drainage system.

SPORTS FIELDS

Sports field surfaces endure high traffic, which if not drained adequately, results in costly and time consuming maintenance. Adequate drainage requires the fast and effective removal of water after a storm.

The use of narrow width **Megaflo**[®] flat panel drain ensures minimal disruption of the existing sports surface with a simple, cost effective installation process.

RETAINING WALLS

Megaflo[®] provides reliable drainage in specialist construction applications such as retaining walls, shotcrete walls and tunnels.

The **Megaflo**[®] drainage system can be utilised vertically or horizontally to remove excess water, preventing the build up of water pressures induced on the structure.

LANDFILL

Landfill leachate and gas collection systems are an integral part of landfill design for lining and capping systems.

The high compressive strength of **Megaflo**[®] under normal and inclined loads makes it the ideal product for a range of landfill drainage applications.

Megaflo[®] is ideally suited for use as collector drains in mining applications. Its high compressive modulus and structural rigidity prevents deflection and the loss of flow capacity under high load or localised settlement.

Megaflo® Ultra is available for the rapid drainage of tailing dams.

GOLF COURSES

The trenchless option of **Megaflo**[®] laid directly onto the subgrade results in significant savings in man-hours and material compared to traditional round pipe.

Regardless of the subgrade soil type, all golf courses can benefit from improved drainage using **Megaflo**[®].













ADVANTAGES OF MEGAFLO® DRAINAGE SYSTEMS

High Crush Strength	The high vertical crush strength means Megaflo [®] can be installed closer to the surface reducing the cost of excavation.
Enhanced Performance	The increased height and rapid response times associated with Megaflo [®] ensures the system outperforms traditional drainage options. The flat pipe construction prevents intrusion of the cover geotextile allowing flow rates to be maintained despite soil confinement pressure.
Cost Effective	The narrow trench width requirement combines rapid installation of the geotextile encapsulated Megaflo® to provide significant cost savings when compared to traditional French drain systems.
Environmentally Friendly	Megaflo [®] is manufactured from recycled HDPE, minimising the carbon footprint of the project.

MEGAFLO® DIMENSIONS

Megaflo [®] Products:						
Product Description	Height	Roll Length				
Megaflo [®] 170	170mm	50m or 100m				
Megaflo [®] 300	315mm	50m or 100m				
Megaflo [®] 450	450mm	50m or 100m				
Megaflo [®] 900	900mm	50m				

FITTINGS

A range of standard **Megaflo®** fittings are available:

- Megaflo[®] couplers are a high strength, secure means to join continuous sections of Megaflo®, inserted beneath the geotextile.
- Connecting to 100mm diameter round pipe is easily achieved using either the Megaflo® side outlet or Megaflo[®] end outlet where required.
- ٠ Megaflo[®] end caps can be fitted to terminations to prevent backfill ingress into the system. Other fittings are available for connecting Megaflo® in various arrangements depending on the application.



Fittings are available to suit all Megaflo[®] sizes:

А	Joiner Coupling	С	Side Outlet
В	End Outlet	D	End Cap

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IMPORTANT NOTICE - DISCLAIMER

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Appendix G: Drainage System As-Builts in Northern Remediation Containment Area



SUB SOIL WATER COLLECTION PIT 1500x1500 PRECAST SEALED PIT WITH CLASS 'D' CONCRETE INFILL LID. REFER TO DETAIL ON DRG No026	Ø100 PVC CONTAMINATED DRAINAGE LINE AT INVERT OF SEWER MAIN - GRADED TO GROUND WATER COLLECTION PIT	The second secon
A A A A A A A A A A A A A A A A A A A	LINE 1 0/50 BEP PREFAT CONCRETE DEFINITION STRAIGHT BACK TAREPER AND COVER AND FRAME. COVER AND F	x x x x x x x x x x x x x x x x x x x



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Appendix H: Figures from Guardhouse Validation Report





PROJECT VALIDATION REPORT GUARDHOUSE SITE PLATYPUS SITE, HIGH STREET NORTH SYDNEY

CLIENT SYDNEY HARBOUR FEDERATION TRUST







APPROVED RO LEGEND SITE BOUNDARY E 325501.665 N 6248326.116

C121007 F004

25.04.18

RR

PROJECT No.

FILE NAME

DATE

DRAWN

COORDINATES IN M.G.A.













Appendix I: Documentation from Torpedo Factory Renewal Validation Report



I1 Figures



















CLIENT SYDNEY HARBOUR FEDERATION TRUST

PROJECT

TITLE MARKER LAYER LEVELS

FIGURE No.











I2 Marker Layer Specifications

Bastion

Bastion 1 x 50m Orange Barrier Mesh

★★★☆☆ 3.3 (4)

BASTION



Compare

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I/N: 0332625





Select your preferred purchase method

 In-Store
 Image: Operation of the store

 Find your item in store
 We are unable to determine your nearest store

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 Set your store

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mastaTEX Orange Hi Vis Geotextile Warning Layer

Product Images







Short Description

FW grade high vis orange geotextile, designed for separating contaminated and non-contaminated soils ie in asbestos remediation applications. Available in a 150m roll, 3m or 6m wide. ADDITIONAL FREIGHT CHARGES MAY APPLY - You may be contacted about freight cost for heavy, bulky or awkward items. Call us for details on 1300 885 364.

Description

mastaTEX Orange is a high quality staple fibre geotextile, designed for separating contaminated and noncontaminated soils. Due to its bright hi vis orange colouring, mastaTEX Orange is most commonly used as a warning layer in areas which are liable to future excavations. mastaTEX orange high vis geotextile provides the same performance as other geotextiles in the mastaTEX[™] range

Applications:

- Segregation of contaminated soil
- Filtration
- Warning Layer



I3 As build Documentation

Sub Base Platypus 120 High St, North Sydney NSW 2060

LANDSCAPE PACKAGE

Drawing Number Drawi

Cover sheet
Legend and Landscape Schedule
Specification
Planting Plan - East
Planting Plan - West
Planting Schedule
Planting Details - 1 of 1
Planting & Wire Trellis Details

1. THIS DRAWING IS COPYRIGHT AND IS THE PROPERTY OF TURF DESIGN STUDIO AND MUST NOT BE USED WITHOUT PERMISSION. 2. THIS DRAWING IS AN UNCONTROLLED COPY UNLESS STAMPED WITH STATUS THIS DRAWING IS A UNCOUNT NOLLED COFF UNLESS STAIDED WITH STATUS THE CONTRACTOR MUST VERIFY ALL DIMENSIONS AND LEVELS ON SITE AS THIS IS
 NOT A SHOP DRAWING.
 THE CONTRACTOR MUST REFER ANY DISCREPANCIES TO THE SUPERINTENDENTS REPRESENTATIVE BEFORE PROCEEDING WITH THE WORK. 5. THE CONTRACTOR MUST NOT SCALE FROM THESE DRAWINGS. USE NOMINATED DIMENSIONS AND LEVELS. DIMENSIONS AND LEVELS. 6. THIS DRAWING MUST BE READ IN CONJUNCTION WITH THE TOTAL CONTRACT PACKAGE INCLUDING DRAWINGS AND SPECIFICATIONS OF ALL CONSULTANTS. IF THE CONTRACTOR INTENDS TO UNDERTAKE ANY EXCAVATION WORK, IT IS THE 7. CONTRACTORS RESPONSIBLITY TO CONTACT 1100 - DIAL BEFORE YOU DIG OR WWW.1100.COM.AU SURVEY INFORMATION

GENERAL NOTES

17/08/2023 AS BUILT 8. TURF DESIGN STUDIO DO NOT WARRANT THE ACCURACY OF SURVEY INFORMATION. Rev Date Description

OM SK Drawn Checked

Project Status **AS BUILT**

Drawing Number **Drawing Title**

Author	Client	Drawing Title
turf	Harbour Trust	Cove
Turf Design Studio : P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale, NSW 2008 Phone (+61 2) 8394 9990 Email: sydney@turfdesign.com	Sub Base Platypus	Drawing Status

ver sheet

BUILT



Scale. @A1 Project No. 2040

Plot Date 17/08/2023 6:29:22 PM Drawing No.



Revision N/A

Sub Base Platypus 120 High St, North Sydney NSW 2060

LANDSCAPE PACKAGE

DRA	WING LIST								
Drawing Number	Drawing Title	Current Revision	Current Revision Date	Current Revision Description	Drawing Number	Drawing Title	Current Revision	Current Revision Date	Current Revision Description
L-P-000	Cover Sheet	K	26/03/2024	AS BUILT	L-P-934	Details - Balustrades & Handrails - Sheet 2	D	26/03/2024	AS BUILT
L-P-011	Legend & Landscape Schedule - Sheet 1	Μ	26/03/2024	AS BUILT	L-P-935	Details - Balustrades & Handrails - Sheet 3	G	26/03/2024	AS BUILT
L-P-012	Legend & Landscape Schedule - Sheet 2	J	26/03/2024	AS BUILT	L-P-950	Details - Softscape - Sheet 1		26/03/2024	AS BUILT
L-P-200	General Arrangement Plan	V	26/03/2024	AS BUILT	L-P-951	Details - Softscape - Sheet 2	F	26/03/2024	AS BUILT
L-P-300	Levels & Drainage Plan	0	26/03/2024	AS BUILT	L-P-952	Details - Softscape - Sheet 3	E	26/03/2024	AS BUILT
L-P-520	Planting Plan - Trees	J	26/03/2024	AS BUILT	L-P-953	Details - Softscape - Sheet 4	С	26/03/2024	AS BUILT
L-P-521	Planting Plan - Understorey and Shrubs	G	26/03/2024	AS BUILT	L-P-960	Details - Rock Platform - Sheet 1	J	26/03/2024	AS BUILT
L-P-530	Planting Schedule	Н	26/03/2024	AS BUILT	L-P-970	Details - Bollard Setout	В	26/03/2024	AS BUILT
L-P-607	Overall Section - Sheet 1	E	26/03/2024	AS BUILT	L-P-1000	Details - Balustrade with Steel Kickrail	В	26/03/2024	AS BUILT
L-P-608	Overall Section - Sheet 2	С	26/03/2024	AS BUILT					
L-P-614	Site Sections - Sheet 5	D	26/03/2024	AS BUILT					
L-P-621	Detail Sections - Sheet 2	С	26/03/2024	AS BUILT					
L-P-625	Detail Sections - Sheet 6	D	26/03/2024	AS BUILT					
L-P-626	Detail Sections - Sheet 7	С	26/03/2024	AS BUILT					
L-P-627	Detail Sections - Sheet 8	E	26/03/2024	AS BUILT					
L-P-900	Details - Paving & Walls - Sheet 1		26/03/2024	AS BUILT					
L-P-901	Details - Paving & Walls - Sheet 2	G	26/03/2024	AS BUILT					
L-P-902	Details - Paving & Walls - Sheet 3	D	26/03/2024	AS BUILT					
L-P-904	Details - Paving & Walls - Sheet 5	F	26/03/2024	AS BUILT					
L-P-906	Details - Paving & Walls - Sheet 7	F	26/03/2024	AS BUILT					
L-P-910	Details - Interfaces	D	26/03/2024	AS BUILT					
L-P-920	Details - Stairs & Handrails - Sheet 1	K	26/03/2024	AS BUILT					
L-P-921	Details - Stairs & Handrails - Sheet 2	Н	26/03/2024	AS BUILT					
L-P-922	Details - Stairs & Handrails - Sheet 3	F	26/03/2024	AS BUILT					
L-P-923	Details - Stairs & Handrails - Sheet 4	D	26/03/2024	AS BUILT					
L-P-924	Details - Stairs & Handrails - Sheet 5	С	26/03/2024	AS BUILT					
L-P-925	Details - Stairs & Handrails - Sheet 6	A	26/03/2024	AS BUILT					
L-P-930	Details - Furniture & Fixtures - Sheet 1	E	26/03/2024	AS BUILT					
L-P-931	Details - Furniture & Fixtures - Sheet 2	G	26/03/2024	AS BUILT					
L-P-933	Details - Balustrades & Handrails - Sheet 1	E	26/03/2024	AS BUILT					

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J 15/12/2023 FOR CONSTRUCTION

D 03/02/2023 IFT Addenda

K 26/03/2024 AS BUILT

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- DR MH DR MH DR MH DR MH DR MH DR MH

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DR MH Drawn Checked

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turf
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Author

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Drawing Title

Sub Base Platypus

Client

se BIM

Cover Sheet

Drawing Status AS BUILT

Scale. @A1 Project No. 2040

Plot Date 26/03/2024 7:37:07 PM Drawing No.



Revision Κ

CODE	SYMBOL	ITEM/PRODUCT	DIMENSION	COLOUR + FINISH	DRAWING NUMBER	SUPPLIER (or equal)	SHOP DRAWINGS	SAMPLES	HOLD/INSPECTION POINTS	NOTES
PAVING									· · · · · · · · · · · · · · · · · · ·	7
P1		Decomposed Granite Paving	Refer to Detail	Cowra Gold	1/L-P-900	Benedict Industries	-	Required	Inspection point: Preparation of subgrade is complete. Hold point: Approval of sample required prior to installation. Minimum size 3m x 3m.	P5 Slip Rating. Decomposed granite bound with soil bond A01 at 2% application rate i accordance with manufactures specification. Do not apply within 1m ra tree.
P2		Sandstone Crazy Paving	Thickness: 40mm Paver size: minimium 300mm and maximum 1200mm	Colour: Brown quarry run. minimal banding. Finish: Gang sawn	3&4/L-P-900	Gosford Quarries	Required	Required	Inspection point: Preparation of subgrade is complete. Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation. Minimum size 3m x 3m.	P5 Slip Rating. If expansion joints are required, they are to be filled in with a silicone p to match Sandstone colour. Contractor to inform Turf if expansion joint required.
P2#		Oversized Sandstone Landing Pavers	Thickness: 40mm Paver size: Refer to details	Colour: Brown quarry run. minimal banding. Finish: Gang sawn	L-P-902, L-P-920, L-P-921 & L-P-922	Gosford Quarries	Required	Required	Inspection point: Preparation of subgrade is complete. Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation. Minimum size 3m x 3m.	P5 Slip Rating. If expansion joints are required, they are to be filled in with a silicone p to match Sandstone colour. Contractor to inform Turf if expansion joint required.
P4		Asphalt Paving	Repair and Reinstate Asphalt Paving to SHFT Specification	To match surrounding asphalt	-	TBD	-	-	Asphalt to be reinstated/made good over machine shed roof. SHFT to confirm asphalt colour and specification.	P5 Slip Rating. Asphalt to be reinstated/made good over machine shed roof. SHFT to asphalt colour and specification.
P5		Random Sandstone Stepping Stones	Thickness: 400mm Paver size: minimium 600mm and maximum 1200mm	Reclaimed flat top boulders from site.	2/L-P-900	-	-	-	Hold point: Boulder selection and final arrangement to be confirmed on site by Turf Design Studio representative.	P5 Slip Rating.
										$\underline{\ } \underline{\ } \ $

WALLS									
RP	Rock Platform	Refer to details	Reclaimed sandstone OR Colour: Brown quarry run. minimal banding. Finish: Gang sawn	L-P-960	Reclaimed sandstone OR Gosford Quarries	Required	Required	 Hold Point: Appoval of construction methodology provided by Stonemason Hold Point: Approval of shop drawings Hold Point: Selection and approval of stone Hold Point: 3m x full depth insitu portion of wall 	-
SE	Galvanised Steel Edging	2.5mm x 100mm	Galvabond	L-P-910	Ideal Edging	-	-		Edging to have smooth shapes are expressed
SL#	Sandstone Log Wall	Refer to details	Reclaimed sandstone OR Colour: Brown quarry run. minimal banding. Finish: Gang sawn	L-P-904 & L-P-905	Reclaimed sandstone OR Gosford Quarries	-	Required	Hold Point: Selection and approval of stone Hold Point: 3m x full length insitu portion of wall	Contractor to advise utilised.
W1	Brick clad blockwork wall	Refer to detail	Recycled Brick to match southern notch	L-P-906	The Brick Pit	-	Required (Approved)	Hold Point: Appoval of 3m full height wall	-
W3	Insitu Concrete Wall	200mm wide 15mm chamfer to front and back face of top of wall	Colour: Natural Concrete colour Finish: Off Form Class 2	L-P-901	-	-	-	Contractor to ensure smooth transitions between raked level changes in top of wall. Expansion joints are to be filled with a silicone product (colour to match wall).	

STAIRS									
KR1	Sandstone Kerb Rail	100mm x 150mm x 300mm	Colour: Brown quarry run. minimal banding. Finish: Gang sawn	L-P-935	Gosford Quarries	-	Required	Hold point: Approval of sample required prior to installation. Hold Point: Appoval of 3m insitu run in between handrail.	Ensure compliance v Sandstone block to b
KR2	Galvanised Steel Kerb Rail	150mm x 6mm	Finish: Hot dipped galvanised stee	L-P-901	-	-	Required	Hold point: Approval of sample required prior to installation. Hold Point: Appoval of 3m insitu run in between handrail.	Ensure compliance v Sandstone block to b
SF1/2/3	Sandstone Stairs	Refer to details	Colour: Brown quarry run. minimal banding. Finish: Gang sawn	L-P-920 to L-P-922	Gosford Quarries	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	Ensure compliance v
SN	Stair Nosing	75mm wide	Product: Rufazel Slip Resistant strips. Colour: Black	L-P-920 to L-P-923	Latham	Required	Required	Hold point: Approval of sample required prior to installation. Hold Point: Appoval of installation of stair nosing insitu on tread.	Ensure compliance v Heat treatment applic surface is clean prior
TI	Tactile indicators	Outer Diameter: 35mm Top Diameter: 25mm Thickness: 5mm Shaft: M6 x 18mm	Product: Urethane classic warning tactile Colour: Black	L-P-902, L-P-920, L-P-921 & L-P-922	DTAC	Required	Required	Hold point: Approval of sample required prior to installation. Hold Point: Appoval of tactile setout prior to installation.	Ensure compliance v No tactiles to be fixed 20mm from edge of

BALUSTRA	LUSTRADES AND HANDRAILS										
B1a	Balustrade	Refer to details	Material: Hot dipped galvanised steel - 300 grade	L-P-933, L-P-934 & L-P-936	Major Engineering Australia	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	Ensure compliance w No site welding - due		
B1b	Balustrade - with integrated handrail	Refer to details	Material: Hot dipped galvanised steel - 300 grade	L-P-933, L-P-934 & L-P-936	Major Engineering Australia	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	Ensure compliance w No site welding due t galvanised fixing cup be fixed together with equivalent) between		
HR1	Handrail - Ramp	Refer to details	Material: Hot dipped galvanised steel - 300 grade	L-P-935	Major Engineering Australia	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	Ensure compliance w No site welding - due galvanised fixing cup be fixed together with equivalent) between		
HR2	Handrail - Stairs	Refer to details	Material: Hot dipped galvanised steel - 300 grade	L-P-920 to L-P-922	Major Engineering Australia	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	Ensure compliance w No site welding - due galvanised fixing cup be fixed together with equivalent) between		

NOTES

ALL STEEL IN ACCORDANCE WITH AS:4312 CORROSIVITY ZONES IN AUSTRALIA • ALL SANDSTONE QUARRY LOCATIONS TO BE CONFIRMED WITH SUPPLIER PRIOR TO ORDERING TO • ENSURE CONSISTENCY BETWEEN SANDSTONE ELEMENTS ACROSS THE PROJECT

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Harbour Trust

Client

Drawing Title Drawing Status

Sub Base Platypus

ite bound with soil bond A01 at 2% application rate in anufactures specification. Do not apply within 1m radius of

are required, they are to be filled in with a silicone product the colour. Contractor to inform Turf if expansion joints are

are required, they are to be filled in with a silicone product the colour. Contractor to inform Turf if expansion joints are

ated/made good over machine shed roof. SHFT to confirm specification.

 \sim

ooth transition between sections to ensure that ovoid sed. e if reclaimed stone or sandstone logs (C Grade) will be

with AS1428.1. be mortared to same slab as P2 paving. with AS1428.1. be mortared to same slab as P2 paving. with AS1428.1.

with AS1428.1. ication - to match concrete stairs on site. Ensure stair to intsalling. with AS1428.1.

ed into mortar joints. Centre of tactile to be a minimium of f paver to prevent cracking.

with AS1428.1. e to highly corrosive environment.

with AS1428.1.

e to highly corrosive environment. Contractor to pre-weld ups to balustrade support plate. Balustrade and handrail to rith appropriate screws. Contractor to apply a silicone (or on cup and handrail to prevent elements rusting together. with AS1428.1.

ue to highly corrosive environment. Contractor to pre-weld ups to handrail support post. Support post and handrail to ith appropriate screws. Contractor to apply a silicone (or n cup and handrail to prevent elements rusting together. with AS1428.1.

ue to highly corrosive environment. Contractor to pre-weld ups to handrail support post. Support post and handrail to with appropriate screws. Contractor to apply a silicone (or en cup and handrail to prevent elements rusting together.

Legend & Landscape Schedule - Sheet 1 AS BUILT

Scale. @A1 Project No. 2040

Plot Date 26/03/2024 7:37:10 PM Drawing No.



se BIM 360

CODE	SYMBOL	ITEM/PRODUCT	DIMENSION	COLOUR + FINISH	DRAWING NUMBER	SUPPLIER (or equal)	SHOP DRAWINGS	SAMPLES	HOLD/INSPECTION POINTS	NOTES
FURNITU	RE AND FI	XINGS								
PT		Picnic Table	Refer to detail	Stone:Reclaimed Sandstone from siteOR Brown quarry run with minimalbanding.Finish: Gang sawnTimber:Australian FSC certified hardwoodFinish: Cabot's Garden FurnitureOilSteel:Hot dipped galvanised steel	L-P-930	Stone Reclaimed stone from site OR Gosford Quarries Timber: MosUrban	Required	Required	Hold point: Approval of shop drawings Hold point: Approval of sample required prior to installation.	-
RSB		Reclaimed Sandstone Boulder	Min 1500mm wide.	-	L-P-621	-	-	-	Hold Point: Boulder selection and final arrangement to be confirmed on site.	Ensure boulder doe surrounding areas is
RSS		Reclaimed Sandstone Seat/Sandstone Log	Approx 570mm x 570mm x 1200mm	-	L-P-931	-	-	-	Hold Point: Turf to select blocks from stockpiled material. Final arrangement to be confirmed on site by Turf Design Studio representative.	Min height 400mm Max height 550mm
CODE	SYMBOL	ITEM/PRODUCT	DIMENSION	COLOUR + FINISH	DRAWING NUMBER	SUPPLIER (or equal)	SHOP DRAWINGS	SAMPLES	HOLD/INSPECTION POINTS	NOTES
SOFTWO	RKS									
0		Existing Site Fill	-	-	-	-	-	-	-	To be compacted to
1		Existing Tree to be Retained	-	-	-					

CODE	SYMBOL	ITEM/PRODUCT	DIMENSION	COLOUR + FINISH	DRAWING NUMBER	SUPPLIER (or equal)	SHOP DRAWINGS	SAMPLES	HOLD/INSPECTION POINTS	NOTES
SOFTWO	RKS									
0		Existing Site Fill	-	-	-	-	-	-	-	To be compacted to
1	0	Existing Tree to be Retained	-	-	-					
1	+	Proposed Tree	-	-	L-P-520, L-P-525 & L-P-530	-	-	-	Hold Point: placement of trees in bags/pots in specifed locations prior to planting	All trees to meet rel
2		Marker Layer: Geogrid open weave	-	Grid spacing to be min 100mm x 100mm	L-P-950 to L-P-953	Geofabrics	-	-	Hold Point: To be approved by Consara prior to installation	Refer to RAP for m
2		Marker Layer: Geofabric 'bidim' non woven	-	-	L-P-950 to L-P-953	Geofabrics	-	-	Hold Point: To be approved by Consara prior to installation	Refer to RAP for m
3		Soil: Horizon B - Benedict Smartmix® 7 Native Garden Sub-Soil Mix	-	Below 300mm	L-P-950 to L-P-953	Benedicts	Required	-	Hold Point: Inspection of marker layer over existing site fill prior to installation of Horizon B. Inspection undertaken by Consara.	
3		Soil: Horizon A - Benedict Smartmix® 6 Native Garden Mix	-	Top 300mm	L-P-950 to L-P-953	Benedicts	Required	-	Hold Point: Inspection of marker layer over existing site fill prior to installation of Horizon A. Inspection undertaken by Consara.	
3		Mulch: Forest Blend	-	75mm deep	L-P-950 to L-P-953	ANL	Required	-	-	-
P3	24	Sandstone Rock Swale	-	Swale Spalls to consist of an ever mix of 200mm - 300mm spalls & 8mm - 20mm spalls.	ນ 2/L-P-951	Benedicts	Required	-	Hold Point: Installation of subsoil drainage	-
P6	S	Sandstone Spall	-	200mm - 300mm sandstone spalls	s 2/L-P-951	Benedicts	Required	-	-	-

NOTES

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Client

Drawing Title Australian Government Sydney Harbour Federation Trust

Sub Base Platypus

s not pose a fall height. Ensure fall from top of boulder to s no more then 920mm.
o engineers specification
evant standards (NATSPEC and AS)
ore information
ore information

Legend & Landscape Schedule - Sheet 2 Drawing Status AS BUILT

Scale. 1:1@A1 Project No. 2040

Plot Date 26/03/2024 7:37:12 PM Drawing No.



BIM 360:

Revision

J



REFER TO L-P-300 FOR ALL LEVELS. LEVELS HAVE BEEN PROVIDED BY USHER & COMPANY SURVEYORS AND ARE AS PER SITE CONDITIONS.



USHER&COMPANY

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PRECISION LANDSCAPES PTY LTD Sub Base Platypus Stage 2

Blant	Flee	Quantitu	Nata /Culestantons
	Size	Quantity	Notes/Substitutions
GROUNDCOVERS AND CLIMBERS			
Mix 1 - Even Mix of below, Qty 485:	-		
Carex appressa	Tube	122	
Ficinia nodosa	Tube	121	
Juncus usitatus	Tube	121	
Lomandra hystrix	140mm	121	
Mix 2 - Even Mix of Below, Qty 109:	140mm	54	
Gannia sieberiana	140mm	54	
Mix 3 - Even Mix of below, Ory 148	Tablutu	35	
Baeckea linifolia	140mm	37	
Banksia Integrifolia 'Roller Coaster'	140mm	37	
Hardenbergia violacea	140mm	37	
Hibbertia scandens	140mm	37	
Mix 4 - Even Mix of below, Qtv 207:			
Correa alba	140mm	41	
Lomandra longifolia 'Verday'	140mm	41	
Melaleuca thymifolia	140mm	41	
Rhagodia spinescens	140mm	42	
Westringia fruiticosa	140mm	42	
Mix 5 - Even Mix of below. Otv 47:	a norm	14	
Chrysocephalum aniculatum 'desert flame'	140mm	47	
Mix 6 - Even Mix of below. Otv 271:	artenini i	-0	
Diagella revoluta	Tube	91	
Hibbertia scandens	140mm	90	
Lomandra hystrix	140mm	90	
Mix 7 - Even Mix of below. Otv 44-	210mm	54	
Cissus antartica	140mm	11	
Grevillea 'Poorinda Roval Mantle'	140mm	11	
Melaleuca thymifolia	140mm	11	
Poa labillardierei	140mm	11	
Mix 8 - Even Mix of below. Otv 576:			
Correa alba	140mm	115	
Isopogona anethifolius	Tube	115	
Lomandra longifolia	140mm	115	
Poa labillardierei	140mm	115	
Ozothamnus diosmifolius	Tube	116	
Mix 9 - Even Mix of below. Otv 38:	1.000		
Diagella revoluta 'Blue Stream'	140mm	19	
Lomandra longifolia	140mm	19	
Mix 10 - Even Mix of below. Otv 330:	2 Horisin	44	
Leotospermum polygalifolium	Tube	66	
Correa alba	140mm	66	
Melaleuca thymifolia	140mm	66	
Rhagodia soinescens	140mm	56	
Westringia fruticosa 'Mundi'	140mm	66	
Mix 11 - Even Mix of below Oty 121-	196010111	00	
Dianella caerulea	Tube	41	
Lomandra longifolia	140mm	40	
Lomandra longifolia 'Verday'	140mm	40	
Mix 12 - Even Mix of below Oty 553	140000	40	
Lentospermum, polygalifolium	Tubo	71	
Banksia spinulosa	140mm	70	
Carnobrotus glaucescens	140mm	70	
Carey appressa	Tube	70	
Hibbertia scandens	140mm	70	
luncus usitatus	Tube	71	
Lomandra longifolia 'Verday'	140mm	70	
Rhandia spinescene	140000	70	
Mix 13 - Even Mix of below Oty 110	140/0/0	10	
Carpobrotus abusessens	110	10	
Carpobiotus glaucescens	Tube	10	
Carex appressa	lube	19	
hubertia scandens	140mm	18	
Inmandra hustalu	edu)	19	
Reading animatica	140mm	18	
knagodia spinescens	140mm	18	
	lotal	3049	-

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- D 11/05/2023 FOR PLANT PROCUREMENT

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Description

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TREES			
Angophora costata	100L	17	
Angophora costata	200L	6	
Acacia longifolia subsp. sophorae Coast Wattle	300mm	24	
Acacia myrtifolia	300mm	21	Pregrow only. If not ready, downsize or sub Acacia longifol
Banksia ericifolia Heath-leaved banksia	45Ltr	6	
Banksia integrifolia	100L	11	
Baeckea linifolia 'Swamp Baeckea'	5L	1	Largest size available.
Cyathea australis Tree Fern	45Ltr	8	
Ficus rubiginosa Port Jackson Fig	4m tall	1	
Kunzea ambigua Tick Bush	300mm	1	
Livistonia australis Cabbage Tree palm	4m tall	3	2m Clear Trunk
Xanthorrhoea australis Grass Tree	ex-ground	14	
	Total	113	

Author
turf
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Phone (+61 2) 8394 9990
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Sub Base Platypus

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Plot Date 26/03/2024 7:37:58 PM Drawing No.



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rall Section - Sheet 1

0 2 4 6 8 10 0 SCALE - 1:50 @ A1 , 1:100 @ A3

Plot Date 26/03/2024 7:38:04 PM Drawing No.



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rall Section - Sheet 2

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Detail Sections - Sheet 2

Scale. 1:20 @A1 Project No. 2040

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il Sections - Sheet 6

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0 200 400 600 800 1000mm SCALE - 1:20 @ A1 , 1:40 @ A3

Plot Date 26/03/2024 7:38:22 PM



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Details - Paving & Walls - Sheet 2

Scale. 1:10@A1 Project No. 2040

Plot Date 26/03/2024 7:38:31 PM Drawing No.



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Details - Interfaces

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t	urf	Harbour Trust	Detail Handr
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TI - FINISH IN LINE WITH INNER EDGE OF HANDRAIL

HR2/2 - GALVANISED STEEL HANDRAIL FIXED TO RHS GALVANISED STEEL SUPPORT POST AND SIDE MOUNTED TO CONCRETE FOOTING. HR2/2 ON BOTH SIDES AT BOTTOM OF SF3 STAIRS. HANDRAIL TO FOLLOW CURVATURE OF STAIR EDGE. REFER TO L-P-923.

SF3 - SANDSTONE STAIRS

50mm TREAD OVERLAP TO ENSURE STRUCTURE IS CONCEALED. EXCEPTION: TOP TREAD: 275mm. REFER TO 2/L-P-922.

- HR2 - GALVANISED STEEL HANDRAIL FIXED TO GALVANISED STEEL SUPPORT POST AND SIDE MOUNTED TO CONCRETE FOOTING. HANDRAIL TO FOLLOW CURVATURE OF STAIR EDGE. REFER TO 5/L-P-922.

TI - FINISH IN LINE WITH INNER EDGE OF HANDRAIL

Plot Date 26/03/2024 7:39:19 PM Drawing No.



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Scale. As indicated @A1 Project No. 2040

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Email: sydney@turfdesign.c

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tails - Stairs & ndrails - Sheet 6 Drawing Status AS BUILT

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AUSTRALIAN HARDWOOD TIMBER BATTEN (50mm x 50mm) TABLE TOP - HOT DIPPED GALVANISED STEEL FIXING PLATE (10mm x 100mm) RECLAIMED SANDSTONE (300mm WIDE) BENCH LEGS P1 - DECOMPOSED GRANITE. REFER 1/L-P-900. BENCH AND TABLE SET IN MASS CONCRETE FOOTING. FOOTING TO HAVE MIN 50mm OF P1 COVER. REFER TO ENGINEERS SPECIFICATION.

Details - Furniture & Fixtures - Sheet 1 AS BUILT

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Revision Ε



RSS - RECLAIMED SANDSTONE SEAT (1)

1:10

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tails - Furniture & tures - Sheet 2 BUILT

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Plot Date 26/03/2024 7:39:25 PM Drawing No.



Revision G



B1a/b - BALUSTRADE ELEVATION TYPICAL (1

NOTE:

1:10

- PANELS RANGE BETWEEN 1016 1508mm. PANELS TO BE • EQUALLY DIVIDED FOR EACH GRADED SECTION OF WALL.
- REFER TO L-P-936 FOR PANEL SETOUT.
- MAINTAIN CONSISTENT 123mm SPACING BETWEEN 12Ø GALV. STEEL RODS.
- ASSUMPTION THAT COREHOLE IS 100mm DIAMETRE. COVER PLATE TO COVER ENTIRE COREHOLE.

	1200 LANDING	4 L-P-933
VARIES	VARIES	
	1:40	

B1b - BALUSTRADE ELEVATION TYPICAL $(\mathbf{2})$

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	20mm THICK GALVANISED STEEL PLATE WITH 2x PREDRILLED HOLES.				
	HR1 - 42Ø x 3mm GALVANISED STEEL CHS HANDRAIL TO AS1428.1.				33
			ł	VARIES	
	12Ø GALVANISED STEEL RODS. —			1:14	
	 12mm x 65mm GALVANISED STEEL FRAME —	* *			
	 MIN 8Ø GALVANISED STEEL ALLEN KEY FIXING TO ALL	1000 BALUSTRADE PANEL HEIGHT 1200 BALUSTRADE HEIGHT FROM FFI			
	20mm THICK GALVANISED STEEL PLATE WITH 2x PREDRILLED HOLES.				
	10 x 110 x 110 GALVANISED STEEL COVER PLATE	8	NMO		4.
	ASSUMPTION: 100mm CORE HOLE CENTERED ON WALL	-			
BALUSTRAI					

	Client Harbour Trust Australian Government Sydney Harbour Federation Trust	Drawing Title Detail Handı
, 35 Wellington St Chippendale,	Sub Base Platypus	Drawing Status AS BUI



READ ALL BALUSTRADES & HANDRAILS DETAILS IN CONJUNCTION WITH SHOP DRAWINGS PROVIDED BY MAJOR ENGINEERING. DETAILS IN THIS PACKAGE DEMONSTRATE DESIGN INTENT OF ELEMENTS ONLY.



ADE ELEVATION TYPICAL - DETAIL

Is - Balustrades & rails - Sheet 1 ILT

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B1a - BALUSTRADE - TYPICAL SECTION (1)1:5

B1b - BALUSTRADE - TYPICAL SECTION 5 1:5

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FRAME - BOTTOM PROFILE.

20mm THICK GALVANISED STEEL

HOLE CENTERED ON WALL

W1 - INSITU CONCRETE WALL.









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HR1 - TYPICAL PLAN 4 1:10



Details - Balustrades & Handrails - Sheet 3



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EXISTING SANDSTONE SEAWALL TO BE RETAINED AND PROTECTED. REFER TO STRUCTURAL ENGINEERS METHODOLOGY.

INDICATIVE SETBACK FROM WALL (1.5 x H).
 ENGINEER TO PROVIDE FINAL SETBACK
 DISTANCE FROM SEA WALL.

tails - Softscape et 4 BUILT



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DEEP STRUCK MORTAR JOINT. REFER 7/L-P-960.

COMPACTED SUBGRADE. REFER TO ENGINEERS SPECIFICATION.

SPECIFICATION.

2

1:20

FIRST TIER SANDSTONE BLOCK TO ACT AS BEDDING COURSE TO ABOVE TIERS. SANDSTONE BLOCK TO BE MOSTLY CONCEALED FROM FACE OF THE ROCK PLATFORM. FIRST TIER SANDSTONE BLOCK SET IN MASS CONCRETE FOOTING. REFER TO ENGINEER

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RP - PLATFORM DETAIL SECTION 1

NOTE: DIMENSIONS ARE FROM FACE OF WALL TO FACE OF BOLLARD. EFFECTIVE SPACE BETWEEN BOLLARDS FACE TO FACE SHOULD NEVER EXCEED 1200mm.

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turf	Harbour Trust	Deta
Turf Design Studio : P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale, NSW 2008 Phone (+61 2) 8394 9990 Email: sydney@turfdesign.com	Sub Base Platypus	Drawing Status

ails - Bollard Setout

BUILT



Scale. 1:50 @A1 Project No. 2040

Plot Date 26/03/2024 7:39:43 PM Drawing No.



В

Revision

Ti-tacTile INDICATORS CENTRED D2# - LARGE FORMAT SANDSTONE PAR TI-tacTile INDICATORS CENTRED D2 - SANDSTONE CRAZY PAVING TI-tacTile INDICATORS CENTRED TI-tacTile INDICATORS CENTRED D2 - SANDSTONE CRAZY PAVING TI-tacTile INDICATORS CENTRED			KR2- KERB P-100 B1b- REFE W3 IN REFE
Market and a set of the	MIN 300 P2# OVERS JDINAL TYPICAL SECTION	600 MI 600 MI TACTILES MIN 1200 LANDING 1200-1400 IZE SANDSTONE PAVER	
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BALUSTRADE.



2 KR2 - STEEL KICK RAIL - TYPICAL SECTION 1:5

Author	Client	Drawing Title
tunf	Harbour Trust	Deta
luit		Stee
Turf Design Studio: P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale, NSW 2008 Bhose (Jeff 2) 8204 0000	Project	Drawing Status
Email: sydney@turfdesign.com	Sub Base Platypus	AS B

Details - Balustrade with Steel Kickrail awing Status AS BUILT

Scale. As indicated @A1 Project No. 2040

Plot Date 26/03/2024 7:39:44 PM Drawing No.



В

Revision

Sub Base Platypus 120 High St, North Sydney NSW 2060

LANDSCAPE PACKAGE

Drawing Number Drawi

Cover sheet
Legend and Landscape Schedule
Specification
Planting Plan - East
Planting Plan - West
Planting Schedule
Planting Details - 1 of 1
Planting & Wire Trellis Details

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Project Status **AS BUILT**

Drawing Number **Drawing Title**

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turf	Harbour Trust	Cove
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Plot Date 17/08/2023 6:29:22 PM Drawing No.



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CODE	SYMBOL	ITEM/PRODUCT	DIMENSION	COLOUR + FINISH	DRAWING NUMBER	SUPPLIER (or equal)	SHOP DRAWINGS	SAMPLES	HOLD/INSPECTION POINTS	NOTES

SOFTWORKS								
-	Drainage Cell	30mm thick	Black -	Atlantis - Flo	Cell -	-	-	Ensure filter fabric is s
-	Existing Tree - to be retained	-	-	-	-	-	-	Refer to specification
-	Mulch	75mm thick	Forest Blend' Course 20-40mm gold - coloured chip, made from 100% recycled grade A wood waste.	ANL	-	Photographic samples	Supplier Certificates Required prior to installation	Keep clear of plant ste
- •	Proposed Tree	-	-		-	Photographic samples	Inspection of setout & specimen quality on site prior to installation	All speciments to Nats
-	Soil Type A	Benedict Smartmix® 6 Native Garden Mix	-	Benedicts	-	Photographic samples	Supplier Certificates Required prior to installation	-
-	Soil Type B	Benedict Smartmix® 7 Native Garden Sub-Soil Mix	-	Benedicts	-	Photographic samples	Supplier Certificates Required prior to installation	-
-	Soil Type C	Coarse Washed Drainage Sand	-	Benedicts	-	Photographic samples	Supplier Certificates Required prior to installation	-

TRELLIS S	/STEM						
-	Wire Rope Trellis System	Varies	Grade 316 stainless steel. Jakob Rope Systems Part no. 10820-0600	L-540	Tensile Design & Required Conctruct - 02 999 3668	Samples to be Supplier Certificates Required prior to installation provided for approval prior to installation.	Supplier to provide Er to steel structure to b

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Email: sydney@turfdesign.com	Sub Base Platypus	AS BI

s securely placed over drainage cell. n for tree protection requirements. stem and tree trunk. tspec specifications. Refer to details. Refer to planting schedules for quantity.

Engineering advice. Fittings to provide adequate tension as required. The fixing be designed.

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Plot Date 17/08/2023 6:29:25 PM Drawing No.



Revision N/A

Scale. @A1 Project No. 2040

PRELIMINARIES 1.

1.01. General The following general conditions should be considered prior to the commencement of landscape works. Commencement of work by The Contractor shall be deemed as proof of The Contractor following conditions and existing condition of site.

The landscape plans should be read in conjunction with the survey, architectural plans, engineering plans, service plans, hydraulic plans, and all other relevant documentation prepared by oth All specification information provided below is supplementary to all relevant codes and Australian Standards.

1.02. Workmanship & expertise

The Contractor must be conversant with all current best practice methods relevant to their profession and work being undertaken.

The Contractor (and subconsultants) must be appropriately experienced with all aspects of the work being undertaken.

All planting, establishment and maintenance work must be carried out by qualified arborist / horticulturist with minimum three years' experience in similar work. It is a requirement that the site have the minimum certificate qualification equivalent to a NSW TAFE Course, with a minimum five years demonstrable experience in similar landscape projects. 1.03. Check / Hold Points

The Contractor must provide minimum 5 working days' notice for the Client's representative to inspect the works for compliance with the design documents. At a minimum, inspections are to check / hold points:

Ground preparation Set out (softworks)

Nursery inspection of plant specimens

Installation of advanced trees (or larger)

Prior to practical completion

During defects period

At handover 1.04. Initial preparation

The Contractor must arrange site meeting with the Client's representative (Landscape Architect) prior to commencement, confirm design intent, review documentation and confirm construction No work shall be carried out until all underground services have been identified and accurately located and pegged by Contractor.

The Contractor is to ensure all appropriate safety provisions are made, including but not limited to traffic control and appropriate PPE for all staff. Location of material stockpiles must be agreed with Client's Representative prior to commencement of work on site.

1.05. Protection of existing adjacent elements

The Contractor must ensure all adjacent pavements, urban elements, planting, etc. are adequately protected prior to commencement of works.

1.06. Protection of existing trees

All existing trees to be retained must be protected in accordance with AS4970 - Protection of Trees on Development Sites.

Do not store or otherwise place bulk materials and harmful materials under of near trees within the tree protection zone. Storage of materials, mixing of materials, vehicle parking, disposal of and refuelling, site office and sheds must not occur within the drip line of any existing trees, or within 5m of any existing trees where the drip line radius is less than 5m. Do not place spoil from excavations against tree trunks, even for short periods. Prevent wind-blown materials such as cement from harming trees and plants.

Particular care must be given to root protection; excavation around existing trees must be undertaken by hand methods, unless otherwise approved by the Client's representative. Machine excavation is to be undertaken no closer than 2m from the trunk of existing trees, or until a root with diameter greater than 30mm is encountered. Hand methods should be used aro

30mm.

Soil within 2m of existing trunks should be lightly cultivated (using hand methods). Soil levels should not exceed 100mm greater than existing.

1.07. Erosion & pollution control

The Contractor must take all proper precautions to prevent the erosion of soil from the subject site. The Contractor must install erosion and sediment control barriers as required, and ensure

throughout construction.

An Erosion control plan that reflects the soil type and erosion characteristics of the site must be prepared for approval by the Client prior to commencement of works. 2. SOIL WORKS

2.01. Materials

Soil Types

Refer to the Landscape Legend for specified soil types for all planting.

2.02. Installation Testing

All testing is to be conducted in accordance with AS1289 Method of testing soils for engineering purposes. Site soil shall be given a pH test prior to modifying to ensure conditions are approp shall be taken in several areas where planting is proposed, and the pH shall be adjusted accordingly with sulphur or lime to suit.

A soil test must be undertaken by SESL Australia (or approved equal) for industrial sites. The successful contractor shall implement the recommendations of this test. Subgrade requirements

Refer to the landscape details for required subgrade levels and cultivation depths. Ensure a thorough breakup of the subgrade into a coarse tilth. Grade subgrades to provide falls to surface to the installation of specified topsoil. The Contractor must arrange for Client's Representative (Landscape Architect) to inspect the preparation of sub grade prior to top soil / wearing courses Drainage

To civil engineers drawings, details and specification. Achieving finished levels

Growing media placed as fill in mass planting beds, and tree pits shall be lightly compacted in maximum 75mm layers so as to minimise degree of future settlement. No work shall be carried

whilst soil is still wet, to avoid compaction of these areas. If future settlement occurs Contractor is to allow for additional fill material to achieve design finished levels. Thickening of mulch layer levels is not acceptable.

3. PLANTING

3.01. Materials

Specimen Quality & Size

All plants must be grown well, healthy, not soft or forced, nor rootbound and weed free. The Contractor must undertake a thorough inspection of all plants at the time of delivery and after they delivery vehicle to identify damaged or defective plants. When damaged stock is found the Installation Contractor must immediately notify the Client's representative and the Supplier of the d date and nature of the damage including photographic evidence. At this stage the Supplier will be liable for replacement of defective stock. The Supplier may choose to have the specimens i await a determination by the Client's representative.

All tree stock must be in accordance with AS 2303 "Tree Stock for Landscape Use".

Species

Plants must be true to species or cultivar as named. Plant substitution is not acceptable unless approved by Client's representative (Landscape Architect).

Stakes & Ties Staking of plants is generally not acceptable. All plant stems must be robust enough to support their own weight. In the event that staking is deemed acceptable, min 3 stakes and ties must be unpainted hardwood, straight, free of knots, and pointed at one end. They shall be 2200x50mmx50mm hardwood, or similar approved. Ties shall be 50mm wide hessian webbing material. Fertilisers

Fertilisers must be slow release fertilisers suitable for the proposed planting types. For native plants, specifically Proteaceae family plants (including Grevillea, Banksia, Hakea spp.), low phose used. Fertiliser rate to manufacturers recommendation.

Mulch Refer to the landscape legend for specified mulch material.

3.02. Installation

Setting Out

All planting set out shall be in accordance with the drawings by a suitably qualified horticulturalist. Any conflicts with services must be identified by the Contractor, and adjustment to the location Client's representative (Landscape Architect). Planting

Planting holes shall be dug to the depth as illustrated in details provided. Base of planting hole shall be loosened to a depth of 150mm and a surface dressing of an approved slow release fer manufacturer's recommendations and worked into loosened soil.

Contractor shall ensure that a 'pond' is not dug into clay sub grade material and that planting holes are free draining. Should Contractor not be satisfied with quality of existing soil into which planting holes are free draining. contractor must immediately the advise Client's representative and await further instructions.

All plant material shall be planted as soon after delivery as possible. Plant containers shall be removed and discarded, and the outer roots gently teased from the soil mass. For all tree specie 10% off perimeter prior to planting to remove circling roots and encourage lateral root development.

All plants shall be thoroughly watered immediately prior to planting. Set plant in the centre of hole and backfill with growing media as specified. Compact the backfilled soil and saturate by har remaining air pockets immediately after planting. On completion of planting works the base of each stem shall finish flush with the soil surface level. Planting on Structure

All raised planters to architects specification.

Planter boxes constructed over a concrete slab shall be built in accordance with the following details provided prior to the issue of a relevant Construction Certificate: a) Ensure soil depths are in accordance with the plans and details provided with the application, including the amended plans required by condition B49. The base of the planter must be scree

movement towards subsoil drainage. Refer to civil engineer's details and specification for subsoil drainage. There are to be no external weep holes. b) A concrete hob or haunch shall be constructed at the internal join between the sides and base of the planter to contain drainage to within the planter.

c) Planters are to be fully waterproofed and sealed internally with a proprietary sealing agent and applied by a qualified and experienced tradesman to eliminate water seepage and staining o planter. All internal sealed finishes are to be sound and installed to manufacturer's directions prior to backfilling with soil. An inspection of the waterproofing and sealing of edges is required by backfilling with soil.

d) Drainage cell must be supplied to the base and sides of the planter to minimize damage to the waterproof seal during backfilling and facilitate drainage. Apply a proprietary brand filter fabr imported soil as specified.

e) Install automated drip irrigation - refer to hydraulic engineer for specification. f) Wire Trellis System refer to landscape schedule.

Mulching

GENERAL NOTES

Mulch shall be spread evenly across all planting beds to the depth specified in the drawings. Mulch must be installed clear of all plant stems / trunks as detailed. There shall be no mixing of s

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ctor's acceptance of the	4. 4.01.	Maintenance General
hers for the development.	•	Maintenance shall mean the care and maintenance of contracted works by accepted landscaping or norticultural practices, ensuring that all plants as well as rectifying any defects that become apparent.
	•	The maintenance period shall be 52 weeks beginning from the approved completion of the specified construction work (Practical Completion). Prior to requesting practical completion, The Contractor shall make good / repair any damaged areas, including but not limited to existing turf, pla All must be returned to a condition equivalent to that prior to commencement of works. Contractor shall leave areas over which Contractor has we representative. Contractor shall be responsible for removal from site of all unwanted material and debris resulting from this work
e foreman / leading hand will	•	A qualified landscape maintenance contractor shall undertake the required landscape maintenance works. Timing of maintenance works shall be but not be limited to: watering of planting and irrigation maintenance:
o occur at the following		 removal of any weed growth or litter/debris; replacing failed plants (at no additional cost to the Client unless due to vandalism or some other reason beyond the Contractors control, a make good areas of soil subsistence or erosion; topping up of mulch; spraying of plants as necessary to combat insects or disease; fertilising with approved fertilisers at correct rates; adjusting ties to stakes;
	•	 pruning in accordance with Australian Standard 4373 "Pruning of Amenity Trees". The Landscape Contractor must keep a logbook of all maintenance activity, and submit with each progress claim as evidence of works undertake On completion of the maintenance period, the landscape works shall be inspected and at the satisfaction of the Superintendent or Client's representation of the Client
on methodology.		
f liquids, machinery repairs		
ound tree roots greater than		
e they are maintained		
priate for planting. Tests		
and subsurface drains, prior s.		
d out on mass planting beds er to achieve design finished		
y have been unloaded off the damage, and record the time, returned immediately or		
be provided. Stakes shall be		
sphorus fertilisers shall be		
ion must be approved by the		
rtiliser added to hole to		
plants are to be installed then		
ies, trim potted root balls by		
nd watering to expel any		
eeded to promote water		
of the external face of the by the Certifier prior to		
ric and backfill with an		
soil and mulch material.		

Author	Client	Drawing Title
turf	Harbour Trust	Spec
Turf Design Studio: P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale, NSW 2008 Phone (+61 2) 8394 9990 Email: sydney@turfdesign.com	Sub Base Platypus	Drawing Status

nts are in optimum growing conditions and appearance at all times,

anting, pavements, kerb and gutter, signage, buildings, services. vorked in a tidy condition and to satisfaction of Client's spread regularly over maintenance period. Duties shall include,

, at discretion of Client's representative);

sentative (Landscape Architect), the responsibility will be signed

cification

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Scale. @A1 Project No. 2040

Plot Date 17/08/2023 6:29:40 PM Drawing No.



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Description



Author	Client	Drawing Title					
turf	Harbour Trust	Plant					
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GROUNDCO	VERS AND CLIMBERS - LEVEL 3						
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CODE	SPECIES	COMMON NAME	HEIGHT AT INSTALL	CENTRES(mm)	POT SIZE	QUANTITY	NOTES
Bli	Baeckea linifolia	Swamp Baeckea	0.2m	500	140mm	21	-
Bi	Banksia integrifolia 'Roller Coaster'	Banksia roller coaster	0.2m	400	150mm	94	-
Bs	Banksia spinulosa 'Birthday Candles'	Banksia Birthday Candles	0.3m	400	140mm	70	-
Bh	Banksia spinulosa 'Honey Pots'	Banksia Honey Pots	0.3m	400	140mm	36	-
Ch	Chrysocephalum apiculatum 'Desert Flame'	Yellow Buttons	0.2m	300	150mm	144	-
Са	Correa alba	White Correa	0.2m	300	150mm	211	-
Cr	Correa reflexa	Native Fuchsia	0.2m	400	140mm	19	-
Dc	Dianella caerulea	Blue Flax-Lily	0.3m	400	150mm	301	-
Go	Goodenia ovata	Hop Goodenia	0.2m	400	140mm	33	-
Gr	Grevillea	Grevillea	0.4m	400	200mm	44	Contractor to confirm exact substitute.
Gg	Grevillea 'Gaudi chaudii'	Gaudi's Ghost	0.2m	400	200mm	30	-
Hs	Hibbertia scandens	Snake Vine	0.3m	N/A	150mm	12	-
Lf	Lambertia Formosa	Mountain Devil	0.3m	500	200mm	54	Contractor to confirm exact substitute.
Lp	Leptospermum polygalifolium	Yellow Tea Tree	0.8m	500	200mm	20	-
Lo	Lomandra confertifolia rubiginosa Crackerjack	Mat Rush	0.2m	300	150mm	190	-
Ls	Lomandra fluviatilis 'Shara'	Lomandra 'Shara'	0.3m	400	140mm	104	-
LI	Lomandra longifolia	Mat Rush	0.4m	500	150mm	79	-
Ln	Lomandra longifolia 'Nyalla'	Mat Rush	0.2m	400	140mm	95	-
Lt	Lomandra longifolia 'Tanika'	Mat Rush	0.2m	300	150mm	47	-
Mt	Melaleuca thymifolia	Honey Myrtle	0.2m	500	150mm	19	-
Рр	Pandorea pandorana	Wonga Wonga Vine	0.3m	N/A	200mm	12	-
Pg	Poa labillardieri	Tussock Grass	0.3m	400	140mm	58	-
Wf	Westringia fruticosa 'Mundi'	Coastal Rosemary	0.3m	400	140mm	38	-

IREES AND S	SHRUBS - LEVEL S						
	SPECIES	COMMON NAME	HEIGHT AT INSTALL	CENTRES	POT SIZE	QUANTITY	NOTES
ACO (2)	Angophora costata	Smooth-barked apple	3m	As shown	200L	1	Client rep to determine placement.
AI	Acacia longifolia	Long- leaved Wattle	1m	As shown	300mm	5	-
BIN	Banksia integrifolia	Coast Banksia	3m	As shown	100L	5	Client rep to determine placement.
Gm	Grevillea moonlight 'gold fever'	Moonlight	1m	As shown	20L	7	-
Xg	Xanthorrhoea glauca	Grass Tree	2m	As shown	Ex-Ground	11	Client rep to select specimens and determine placement

Please note: In addition to the specification (where provided) the following controls will apply to plant material brought to site. Where any perceived conflict arises the listed items below are to prevail: 1. The head contractor must place the plant order for the whole site within 2 weeks of signing the head contract to ensure the plant nursery has enough time to source and grow on plant stock. 2. The Genus and Species nominated must be delivered to site in the total quantity specified. Varieties and hybrids of the same genus are not a suitable substitute. 3. Unapproved substitutes will be defected and require replacement.

4. Unapproved reductions to the specified pot sizes will be defected and require replacement, or where approved by client, a cost credit will be applied.
 5. Acceptance of plant stock is wholly at the discretion of the client. Plants deemed to be unsuitable for any reason (damage, root girdling, poor health, no vigorous, yellowing leaves, insect

predation, water stress... etc.) can be rejected by the client and will require replacement at the contractors expense.

6. Plants that are deemed too small for the pot size nominated (as determined by the client and/or the clients representative) will be defected and require replacement.

7. It is incumbent upon the contractor to maintain appropriate quality controls to minimise costly and timely defects rectification. 8. Liaison with the plant nursery, inspecting and receiving plant stock must be undertaken by a qualified horticulturist with a minimum of 5 years practical experience on construction projects. The name and qualifications of the horticulturist must be submitted

for approval within 2 weeks of the head contract being award. The client may accept or reject the submission and the contractor must seek an alternative horticulturist to the satisfaction of the client. Where not under the employ of the contractor, the

contractor must hire a horticulturist for the project. The horticulturist must liaise with the client and clients' representative on all plant related issues and be present at any site inspection during soil, irrigation, and plant installation.

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Author	Client	Drawing Title
turf	Harbour Trust	Plan
Turf Design Studio: P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale, NSW 2008 Phone (+61 2) 8394 9990 Email: sydney@turfdesign.com	Sub Base Platypus	Drawing Status

nting Schedule

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Platypus		

Australian Government Sydney Harbour Federation Trust

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Planting Details - 1 of 1

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Revision

N/A



MUST NOT BE USED WITHOUT PERMISSION. 2. THIS DRAWING IS AN UNCONTROLLED COPY UNLESS STAMPED WITH STATUS THE CONTRACTOR MUST VERIFY ALL DIMENSIONS AND LEVELS ON SITE AS THIS IS 3. NOT A SHOP DRAWING. NOT A SHOP DRAWING.
 THE CONTRACTOR MUST REFER ANY DISCREPANCIES TO THE SUPERINTENDENTS REPRESENTATIVE BEFORE PROCEEDING WITH THE WORK. 5. THE CONTRACTOR MUST NOT SCALE FROM THESE DRAWINGS. USE NOMINATED DIMENSIONS AND LEVELS. THIS DRAWING MUST BE READ IN CONJUNCTION WITH THE TOTAL CONTRACT PACKAGE INCLUDING DRAWINGS AND SPECIFICATIONS OF ALL CONSULTANTS. IF THE CONTRACTOR INTENDS TO UNDERTAKE ANY EXCAVATION WORK, IT IS THE 7. CONTRACTORS RESPONSIBLITY TO CONTACT 1100 - DIAL BEFORE YOU DIG OR WWW.1100.COM.AU

SURVEY INFORMATION

_{N/A} 17/08/2023 AS BUILT 8. TURF DESIGN STUDIO DO NOT WARRANT THE ACCURACY OF SURVEY INFORMATION. Rev Date Description

OM SK Drawn Checked

REFER TO L-540 - 3 FOR CABLE DETAILS.



75mm MULCH - REFER TO LEGEND

SUBSURFACE FULLY AUTOMATED DRIP IRRIGATION SYSTEM TO HYDRAULIC ENGINEERS - DETAILS.

300mm MIN. SOIL MIX A -REFER TO LEGEND

GEOFABRIC 110-80 micron INSTALLED WET BETWEEN - DRAIJNAGE FILLER AND SOIL MIX A. REFER TO MANUFACTURERS SPECIFICATION. DRAINAGE FILTER - COURSE SAND. AS PER MANUFACTURERS SPECIFICATION

GEOFABRIC INSTALLED WET OVER 'DrainEZE' DRAINAGE - HOLE SCREEN. REFER TO MANUFACTURERS SPECIFICATION





PLANTING ON STRUCTURE - QUATRO PLANTER WITH WIRE CABLES 3 1:10



A.ul		
Author	Client	Drawing Title
tunf	Harbour Trust	Plantin
		Details
NSW 2008 Phone (+61.2) 8394 9990	Project	Drawing Status
Email: sydney@turfdesign.com	Sub Base Platypus	AS BUI
		1

ng & Wire Trellis **L**T

• **REFER TO ARCHITECTS**

DETAILS DRAWN TO

MANUFACTURERS

& CONSTRUCT).

SPECIFICATION AND DETAILS

SPECIFICATION (TENSILE DESIGN

FOR RAISED PLANTERS.

GENERAL NOTE:

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Scale. As indicated @A1 Project No. 2040

Plot Date 17/08/2023 6:30:06 PM Drawing No.

L-540

Revision N/A

SUB BASE PLATYPUS 120 HIGH STREET, NORTH SYDNEY - FORESHORE PARK CIVIL ENGINEERING PLANS



SITE LOCALITY PLAN



REV.	DESCRIPTION	DESIGN	DRAWN	CHECKED	VERIFIED	DATE	CLIENT	LANDSC
01	ISSUED FOR 80% CO-ORDINATION	E.B.	W.D.	B.W.S.	C.N.	28/07/23		
02	ISSUED FOR 100% CO-ORDINATION	E.B.	W.D.	D.H.	C.N.	09/08/23		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Stu
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	19/10/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)83
В	REVISIONS AS CLOUDED	E.B.	M.B.	D.H.	C.N.	15/11/23	P +61 8736 9000 W taylorau.com.au	Email: sydney@
С	AS BUILT	E.B.	W.D.	D.H.		28/03/24	VERIFICATION SIGNATURE HAS BEEN ADDED	NORTHE

	DRAWING SCHEDULE								
DRG No.	DRAWING TITLE								
C1.01	COVER SHEET, DRAWING SCHEDULE AND SITE LOCALITY PLAN								
C1.11	CONSTRUCTION SPECIFICATION NOTES								
C2.01	STORMWATER AND GENERAL ARRANGEMENT PLAN								
C2.11	STORMWATER PIPE LONGITUDINAL SECTION SHEET 1								
C2.21	MC01 LONGITUDINAL SECTION SHEET 1 OF 1								
C2.31	MC01 CROSS SECTIONS SHEET 1								
C2.32	MC01 CROSS SECTIONS SHEET 2								
	RW01 LONGITUDINAL SECTION SHEET 1 OF 1								
C2.42	RW02 LONGITUDINAL SECTION SHEET 1 OF 1								
C3.01	DETAILS SHEET 1								
C3.02	DETAILS SHEET 2								
C4.01	BULK EARTHWORKS PLAN								

SOURCE : MAPS.SIX.NSW.GOV.AU (©2023)





ENGINEERING CERTIFICATION NOTES

- 1. TO CERTIFY THE CONSTRUCTED CIVIL WORKS, A QUALIFIED EXPERIENCED ENGINEER IS TO VISIT THE SITE TO OBSERVE CONSTRUCTION TECHNIQUES AND VARIOUS ELEMENTS THAT MAY BE CONCEALED WHEN THE WORKS ARE COMPLETE.
- 2. THIS SPECIFICATION ALLOWS FOR CERTIFICATION OF WORKS CONTROLLED BY A PRIVATE CERTIFIER FOR LAND DEVELOPMENT WORKS. THIS SPECIFICATION DOES NOT COVER CERTIFICATION REQUIREMENTS FOR AUTHORITIES SUCH AS COUNCIL, RMS OR OFFICE OF WATER. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE AND PROVIDE ALL PROJECT SPECIFIC CONSTRUCTION COMPLIANCE (WORKS AS EXECUTED) INFORMATION TO THE SATISFACTION OF THE STAKEHOLDER / AUTHORITY, DISCREPANCIES BETWEEN THIS SPECIFICATION AND SPECIFICATIONS OF OTHER EXTERNAL STAKEHOLDERS / AUTHORITIES IS TO BE REPORTED TO THE SUPERINTENDENT FOR CLARIFICATION.
- 3. THE CONTRACTOR IS TO AGREE WITH THE ENGINEER AN APPROPRIATE SITE VISIT SCHEDULE AND FEE ARRANGEMENT PRIOR TO COMMENCEMENT OF THE WORKS. THE CONTRACTOR SHALL ENSURE THAT THE ENGINEER CAN SAFELY ACCESS ALL CIVIL ELEMENTS TO BE REVIEWED. SITE VISITS ARE CONDUCTED DURING NORMAL BUSINESS HOURS. WE REQUIRE TWO (2) WORKING DAY NOTICE FOR ANY SITE VISIT.
- 4. THE CONTRACTOR SHALL PROVIDE SURVEYED LEVELS, PREPARED BY A QUALIFIED SURVEYOR FOR SUBGRADE, SUB-BASE COURSE, BASE COURSE AND WEARING COURSE.
- 5. THE CONTRACTOR SHALL PROVIDE WORKS AS EXECUTED (WAE) DOCUMENTATION PREPARED BY A QUALIFIED PRACTICING SURVEYOR. THE WAE DRAWINGS SHALL CLEARLY SHOW, STORMWATER GRATE/ COVER LEVELS, STORMWATER PIT INVERT LEVELS AND CORRESPONDING INVERT LEVELS OF ANY INCOMING OR OUTGOING PIPES, DIAMETER OF ALL PIPES, DIMENSIONS AND VOLUME OF ON-SITE DETENTION FACILITIES, INVERT LEVELS OF ORIFICE PLATES, OVERFLOW WEIRS, BASE OF TANK FINISHED LEVELS OF PAVEMENTS. THE WAE SHALL SHOW WHERE THE SIZE OR ALIGNMENT OF CIVIL ENGINEERING ELEMENTS WHEN THEY DEVIATE FROM THE DESIGN DOCUMENTATION.
- 6. THE WAE DRAWINGS SHALL BE STAMPED WITH THE FOLLOWING STATEMENT "THESE WAE DRAWINGS HAVE BEEN PREPARED BY [COMPANY NAME] AND ARE A TRUE AND ACCURATE REPRESENTATION OF THE CONSTRUCTED WORKS". EACH DRAWING SHALL BE SIGNED AND DATED BY THE SURVEYOR WHO PREPARED THE DRAWINGS.

THESE WAE DRAWINGS HAVE BEEN PREPARED BY [COMPANY NAME] AND ARE A TRUE AND ACCURATE REPRESENTATION OF THE CONSTRUCTED WORKS.

SIGNED. DATE..

NAME

POSITION.

- 7. WAE SHALL BE PROVIDED IN BOTH AUTOCAD AND PDF FORMAT. NORTHROP CONSULTING ENGINEERS WILL PROVIDE ENGINEERING PLANS TO THE CONTRACTOR IN AUTOCAD FORMAT TO AID PREPARATION OF WAE DOCUMENTATION.
- 8. CONTRACTOR IS TO UNDERTAKE A CCTV INSPECTION OF ALL STORMWATER DRAINAGE PIPELINES AND PROVIDE TO THE ENGINEER FOR APPROVAL.
- THE CONTRACTOR SHALL PROVIDE ALL RELEVANT TEST CERTIFICATES PROGRESSIVELY THROUGHOUT THE DURATION OF THE WORKS. ALL TEST CERTIFICATES SHALL BE PREPARED BY A NATA REGISTERED LABORATORY. TEST CERTIFICATES ARE REQUIRED FOR PROOF ROLLING, SUBGRADE COMPACTION, COMPACTION OF PAVEMENT LAYERS, COMPACTION OF FILLING OPERATIONS, CONCRETE SLUMP TEST, AND CONCRETE STRENGTH TESTS. THE CONTRACT SHALL PROVIDE ALL RELEVANT VALIDATIONS BY A GEOTECHNICAL ENGINEER FOR ALL IMPORTED FILL
- 10. EACH TEST CERTIFICATE WILL NOMINATE THE DATE AND TIME OF THE TEST AND PROVIDE A LOCATION OF WHERE THE TEST SAMPLE WAS TAKEN FROM.
- 11. THE CONTRACTOR SHALL ARRANGE FOR THE ENGINEER TO CONDUCT A FINAL VISIT TO REVIEW OF THE CONSTRUCTED WORKS. THIS WILL REVIEW WILL NOT TAKE PLACE UNTIL THE WAE DOCUMENTATION AND RELEVANT TEST CERTIFICATES HAVE BEEN RECEIVED.
- 12. IF DEFECTIVE OR INCOMPLETE WORK IS FOUND DURING THE FINAL INSPECTION ANOTHER INSPECTION MAY BE REQUIRED AT THE CONTRACTORS EXPENSE TO VERIFY THE RECTIFICATION WORKS HAVE BEEN COMPLETED.

SAFETY IN DESIGN

EXISTING SERVICES

 CONTRACTOR TO BE AWARE EXISTING SERVICES ARE LOCATED WITHIN THE SITE. LOCATION OF ALL SERVICES TO BE VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING WORKS. CONTRACTOR TO CONFIRM WITH RELEVANT AUTHORITY REGARDING MEASURES TO BE TAKEN TO ENSURE SERVICES ARE PROTECTED OR PROCEDURES ARE IN PLACE TO DEMOLISH AND/OR RELOCATE.

EXISTING STRUCTURES

CONTRACTOR TO BE AWARE EXISTING STRUCTURES MAY EXIST WITHIN THE SITE. TO PREVENT DAMAGE TO EXISTING STRUCTURE(S) AND/OR PERSONNEL, SITE WORKS TO BE CARRIED OUT AS FAR AS PRACTICALLY POSSIBLE FROM EXISTING STRUCTURE(S).

EXISTING TREES

2. CONTRACTOR TO BE AWARE EXISTING TREES EXIST WITHIN THE SITE WHICH NEED TO BE PROTECTED. TO PREVENT DAMAGE TO TREES AND/OR PERSONNEL, SITE WORKS TO BE CARRIED OUT AS FAR AS PRACTICALLY POSSIBLE FROM EXISTING TREES. ADVICE NEEDS TO BE SOUGHT FROM ARBORIST AND/OR LANDSCAPE ARCHITECT ON MEASURES TO PROTECT TREES.

GROUNDWATER

3. CONTRACTOR TO BE AWARE GROUND WATER LEVELS ARE CLOSE TO EXISTING SURFACE LEVEL. TEMPORARY DE-WATERING MAY BE REQUIRED DURING CONSTRUCTION WORKS.

EXCAVATIONS

4. DEEP EXCAVATIONS DUE TO STORMWATER DRAINAGE WORKS IS REQUIRED. CONTRACTOR TO ENSURE SAFE WORKING PROCEDURES ARE IN PLACE FOR WORKS. ALL EXCAVATIONS TO BE FENCED OFF AND BATTERS ADEQUATELY SUPPORTED TO APPROVAL OF GEOTECHNICAL ENGINEER

GROUND CONDITIONS

- 5. CONTRACTOR TO BE AWARE OF THE SITE GEOTECHNICAL CONDITIONS. REFER TO GEOTECHNICAL REPORT FOR DETAILS.
- HAZARDOUS MATERIALS
- 6. EXISTING ASBESTOS PRODUCTS AND CONTAMINATED MATERIAL MAY BE PRESENT ON SITE. CONTRACTOR TO ENSURE ALL HAZARDOUS MATERIALS ARE IDENTIFIED PRIOR TO COMMENCING WORKS. SAFE WORKING PRACTICES AS PER RELEVANT AUTHORITY TO BE ADAPTED AND APPROPRIATE PERSONAL PROTECTIVE EQUIPMENT (PPE) TO BE USED WHEN HANDLING ALL HAZARDOUS MATERIALS.

CONFINED SPACES

7. CONTRACTOR TO BE AWARE OF POTENTIAL HAZARDS DUE TO WORKING IN CONFINED SPACES SUCH AS STORMWATER PITS, TRENCHES AND/OR TANKS. CONTRACTOR TO PROVIDE SAFE WORKING METHODS, USE APPROPRIATE PPE WHEN ENTERING CONFINED SPACES AND BE LICENCED.

MANUAL HANDLING

8. CONTRACTOR TO BE AWARE MANUAL HANDLING MAY BE REQUIRED DURING CONSTRUCTION. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ENSURE MANUAL HANDLING PROCEDURES AND ASSESSMENTS ARE IN PLACE PRIOR TO COMMENCING WORKS.

WATER POLLUTION

9. CONTRACTOR TO ENSURE APPROPRIATE MEASURE ARE TAKEN TO PREVENT POLLUTANTS FROM CONSTRUCTION WORKS CONTAMINATING THE SURROUNDING ENVIRONMENT.

SITE ACCESS/EGRESS

- 10. CONTRACTOR SHALL COMPLY WITH ALL STATUTORY AND INDUSTRIAL REQUIREMENTS FOR PROVISION OF A SAFE WORKING ENVIRONMENT INCLUDING TRAFFIC CONTROL.
- 11. CONTRACTOR IS TO BE AWARE SITE WORKS OCCUR IN CLOSE PROXIMITY TO FOOTPATHS AND ROADWAYS. CONTRACTOR TO ERECT APPROPRIATE BARRIERS AND SIGNAGE TO PROTECT SITE PERSONNEL AND PUBLIC. 12. CONTRACTOR SHALL ENSURE THAT AT ALL TIMES ACCESS TO ALL BUILDINGS
- ADJACENT TO THE WORKS IS NOT DISRUPTED. 13. WHERE NECESSARY, CONTRACTOR SHALL PROVIDE SAFE PASSAGE OF

VEHICLES AND/OR PEDESTRIANS THROUGH OR BY THE SITE. VEHICLE MOVEMENT

14. CONTRACTOR TO SUPPLY AND COMPLY WITH TRAFFIC MANAGEMENT PLAN AND PROVIDE ADEQUATE SITE TRAFFIC CONTROL INCLUDING A CERTIFIED TRAFFIC MARSHALL TO SUPERVISE VEHICLE MOVEMENTS WHERE NECESSARY.

SITE WORKS

- ALL WORKS TO BE IN ACCORDANCE WITH LOCAL RELEVANT COUNCIL REQUIREMENTS, SPECIFICATIONS, AUSTRALIA STANDARDS, CONFLICT SHALL BE REFERRED TO THE SUPERINTENDENT FOR DIRECTION.
- 2. THE CONTRACTOR IS TO DESIGN, OBTAIN APPROVALS AND CARRY OUT REQUIRED TEMPORARY TRAFFIC CONTROL PROCEDURES DURING CONSTRUCTION IN ACCORDANCE WITH RMS AND LOCAL COUNCIL REGULATIONS AND REQUIREMENTS.
- 3. THE CONTRACTOR IS TO OBTAIN ALL AUTHORITY APPROVALS AS REQUIRED. 4. RESTORE ALL PAVED, COVERED, GRASSED AND LANDSCAPED AREAS TO THEIR
- ORIGINAL CONDITION ON COMPLETION OF WORKS. 5. ON COMPLETION OF ANY TRENCHING WORKS, ALL DISTURBED AREAS SHALL BE RESTORED TO THEIR ORIGINAL CONDITION. INCLUDING KERBS. FOOTPATHS, CONCRETE AREAS, GRAVEL, GRASSED AREAS AND ROAD PAVEMENTS.
- 6. THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR.
- 7. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO LODGMENT OF TENDER AND ON SITE WORKS.
- THIS PLAN IS TO BE READ IN CONJUNCTION WITH THE ENGINEERING PLANS, AND ANY OTHER PLANS OR WRITTEN INSTRUCTIONS THAT MAY BE ISSUED RELATING TO DEVELOPMENT OF THE SUBJECT SITE.
- 9. ANY EXISTING TREES WHICH FORM PART OF THE FINAL LANDSCAPING PLAN SHALL BE PROTECTED FROM CONSTRUCTION ACTIVITIES BY: A) PROTECTING THEM WITH BARRIER FENCING OR SIMILAR MATERIALS INSTALLED OUTSIDE THE DRIP LINE.
- B) ENSURING THAT NOTHING IS NAILED TO THEM.
- C) PROHIBITING PAVING, GRADING, SEDIMENT WASH OR PLACING OF STOCKPILES WITHIN THE DRIP LINE EXCEPT UNDER THE FOLLOWING CONDITIONS:
- D) ENCROACHMENT ONLY OCCURS ON ONE SIDE AND NO CLOSER TO THE TRUNK THAN EITHER 1.5 METRES OR HALF THE DISTANCE BETWEEN THE OUTER EDGE OF THE DRIP LINE AND THE TRUNK, WHICHEVER IS GREATER.
- E) A DRAINAGE SYSTEM THAT ALLOWS AIR AND WATER TO CIRCULATE THROUGH THE ROOT ZONE (E.G. A GRAVEL BED) IS PLACED UNDER ALL FILL LAYERS OF MORE THAN 300 MILLIMETRES DEPTH.
- F) CARE IS TAKEN NOT TO CUT ROOTS UNNECESSARILY NOR TO COMPACT THE SOIL AROUND THEM.
- 10. DO NOT OBTAIN DIMENSIONS BY SCALING THE DRAWINGS.
- 11. IN CASE OF DOUBT OR DISCREPANCY REFER TO SUPERINTENDENT FOR CLARIFICATION OR CONFIRMATION PRIOR TO THE COMMENCEMENT OF CONSTRUCTION.
- 12. WHERE NEW WORKS ABUT EXISTING THE CONTRACTOR SHALL ENSURE THAT A SMOOTH EVEN PROFILE, FREE FROM ABRUPT CHANGES IS OBTAINED.
- 13. MAKE SMOOTH TRANSITION TO EXISTING FEATURES AND MAKE GOOD WHERE JOINED.
- 14. THESE PLANS SHALL BE READ IN CONJUNCTION WITH ALL APPROVED DRAWINGS AND SPECIFICATIONS PREPARED BY OTHER PROJECT CONSULTANTS.
- 15. TRENCHES THROUGH EXISTING ROAD AND CONCRETE PAVEMENTS SHALL BE SAWCUT TO FULL DEPTH OF CONCRETE AND A MIN 50mm IN BITUMINOUS

RE∖	/. DESCRIPTION	DESIGN	IDRAWN	CHECKED	VERIFIED	DATE	CLIENT	LANDSCAPE ARCHITECT
01	ISSUED FOR 80% CO-ORDINATION	E.B.	L.M.	B.W.S.	C.N.	28/07/23		- Level
02	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Studio : P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippend.
Α	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	19/10/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	NSW 2008 Phone (+61 2)8394 9990
В	REVISIONS AS CLOUDED	E.B.	M.B.	D.H.	C.N.	15/11/23	P +61 8736 9000 W taylorau.com.au	
С	AS BUILT	E.B.	W.D.	D.H.		28/03/24	VERIFICATION SIGNATURE HAS BEEN ADDED	NORTHROP CONSULTING ENGINEERS PTY LTD.

PAVING.

EXISTING SERVICES

- 1. All UTILITY SERVICES INDICATED ON THE DRAWINGS ORIGINATE FROM SUPPLIED DATA, THEREFORE THEIR ACCURACY AND COMPLETENESS IS NOT GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO DETERMINE AND CONFIRM THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK, ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.
- 2. CARE TO BE TAKEN WHEN EXCAVATION NEAR EXISTING SERVICES. NO MECHANICAL EXCAVATIONS ARE TO BE UNDERTAKEN OVER COMMUNICATION, GAS OR ELECTRICAL SERVICES. HAND EXCAVATION ONLY IN THESE AREAS.
- THE CONTRACTOR SHALL PROTECT AND MAINTAIN ALL EXISTING SERVICES 3. THAT ARE TO BE RETAINED IN THE VICINITY OF THE PROPOSED WORKS. ANY AND ALL DAMAGE TO THESE SERVICES AS A RESULT OF THESE WORKS SHALL BE REPAIRED BY THE CONTRACTOR UNDER THE DIRECTION OF THE SUPERINTENDENT, AND AT NO EXTRA COST.
- 4. THE CONTRACTOR SHALL ALLOW IN THE PROGRAM FOR ADJUSTMENT (IF REQUIRED) OF EXISTING SERVICES IN AREAS AFFECTED BY WORKS.
- 5. THE CONTRACTOR SHALL ALLOW IN THE PROGRAM FOR THE CAPPING OFF EXCAVATION AND REMOVAL (IF REQUIRED) OF EXISTING SERVICES IN THE AREA AFFECTED BY WORKS UNLESS DIRECTED OTHERWISE ON THE DRAWINGS OR BY THE SUPERINTENDENT.
- THE CONTRACTOR SHALL ENSURE THAT AT ALL TIMES SERVICES TO ALL BUILDINGS NOT AFFECTED BY THE WORKS ARE NOT DISRUPTED.
- 7. PRIOR TO COMMENCEMENT OF ANY WORKS THE CONTRACTOR SHALL GAIN APPROVAL OF THE PROGRAM FOR THE RELOCATION AND/OR CONSTRUCTION OF TEMPORARY SERVICES AND FOR ANY ASSOCIATED INTERRUPTION OF SUPPLY.
- THE CONTRACTOR SHALL CONSTRUCT TEMPORARY SERVICES TO MAINTAIN EXISTING SUPPLY TO BUILDINGS REMAINING IN OPERATION DURING WORKS TO THE SATISFACTION AND APPROVAL OF THE SUPERINTENDENT. ONCE DIVERSION IS COMPLETE AND COMMISSIONED THE CONTRACTOR SHALL REMOVE ALL MAKE GOOD TO THE SATISFACTION OF THE SUPERINTENDENT

EARTHWORKS

- 1. AT THE COMMENCEMENT OF THE FILLING OPERATION FOR BULK EARTHWORKS, A GEOTECHNICAL ENGINEER IS TO VISIT THE SITE AND CONFIRM THE SUITABILITY OF THE METHODOLOGY OF ACHIEVING THE REQUIRED BUILDING PLATFORMS AND COMPACTION REQUIREMENTS. SUBSEQUENTLY, THE HEAD CONTRACTOR IS TO CONFIRM, IN WRITING TO THE DESIGNING CIVIL AND STRUCTURAL ENGINEERS, THAT THE METHODOLOGY APPROVED AT THE TIME OF THE GEOTECHNICAL ENGINEERS VISIT WAS MAINTAINED DURING ALL BULK EARTHWORKS PROCESS.
- 2. STRIP TOPSOIL, VEGETABLE MATTER AND RUBBLE TO EXPOSE NATURALLY OCCURRING MATERIAL AND STOCKPILE ON SITE AS DIRECTED BY THE SUPERINTENDENT
- 3. WHERE FILLING IS REQUIRED TO ACHIEVE DESIGN SUBGRADE, PROOF ROLL EXPOSED NATURAL SURFACE WITH A MINIMUM OF TEN PASSES OF VIBRATING ROLLER (MINIMUM STATIC WEIGHT OF 10 TONES) IN THE PRESENCE OF THE SUPERINTENDENT.
- 4. ALL SOFT, WET OR UNSUITABLE MATERIAL IS TO BE REMOVED AS DIRECTED BY THE SUPERINTENDENT AND REPLACED WITH APPROVED MATERIAL SATISFYING THE REQUIREMENTS LISTED BELOW.
- 5. PROVIDE CERTIFICATES VERIFYING THE QUALITY OF IMPORTED MATERIAL FOR THE SUPERINTENDENTS APPROVAL.
- ALL FILL MATERIAL SHALL BE PLACED IN MAXIMUM 200mm THICK LAYERS AND COMPACTED AT OPTIMUM MOISTURE CONTENT (+ OR - 2%) TO ACHIEVE A DRY DENSITY DETERMINED IN ACCORDANCE WITH AS1289 E3.1 OF NOT LESS THAN THE FOLLOWING STANDARD MINIMUM DRY DENSITY IN ACCORDANCE WITH AS1289 E1.1:

LOCATION	COMPACTION REQUIREMENTS
UNDER BUILDING SLABS	98% STANDARD MAXIMUM DRY DENSITY (SMDD)
LANDSCAPING AREAS	95% SMDD
ROADS & PAVED AREAS	98% SMDD

- TESTING OF THE SUBGRADE FOR BUILDINGS SHALL BE CARRIED OUT BY AN 7. APPROVED NATA REGISTERED LABORATORY AND IN ACCORDANCE WITH THE LATEST VERSION OF AS3798 - FOR BUILDING TYPE 1 OPERATIONS
- ALLOW FOR THE COMPACTION TESTING BY NATA REGISTERED LABORATORY FOR PLATFORMS AND FILL LAYERS. IN ACCORDANCE WITH THE LATEST VERSION OF AS3798 - FOR TYPE 1 OPERATIONS (MINIMUM 3 TESTS PER LAYER). WHERE TEST RESULTS ARE BELOW THE SPECIFIED COMPACTION. RECOMPACT
- AND RETEST UNTIL SPECIFIED COMPACTION STANDARD IS ACHIEVED. 10. ALLOW FOR EXCAVATION IN ALL MATERIALS AS FOUND U.N.O. NO ADDITIONAL
- PAYMENTS WILL BE MADE FOR EXCAVATIONS IN WET OR HARD GROUND. 11. WHERE THERE IS INSUFFICIENT EXCAVATED MATERIAL SUITABLE FOR FILLING OR SUBGRADE REPLACEMENT. THE CONTRACTOR IS TO ALLOW TO IMPORT FILL. IMPORTED FILL SHALL COMPLY WITH THE FOLLOWING: A) MAXIMUM SIZE 50mm. PASSING 75 MICRON SIEVE (<25%).
- B) PLASTICITY INDEX BETWEEN 2-15% AND CBR >8.
- C) FREE FROM ORGANIC AND PERISHABLE MATTER.
- 12. REFER TO THE GEOTECHNICAL REPORT FOR GENERAL REQUIREMENTS ON SITE PREPARATION AND RE-USE OF EXISTING SITE MATERIAL AS ENGINEERED FILL.
- 13. THE CONTRACTOR SHALL PROGRAM THE EARTHWORKS OPERATION SO THAT THE WORKING AREAS ARE ADEQUATELY DRAINED DURING THE PERIOD OF CONSTRUCTION. THE SURFACE SHALL BE GRADED AND SEALED OFF TO REMOVE DEPRESSIONS. ROLLER MARKS AND SIMILAR WHICH WOULD ALLOW WATER TO POND AND PENETRATE THE UNDERLYING MATERIAL. ANY DAMAGE RESULTING FROM THE CONTRACTOR NOT OBSERVING THESE REQUIREMENTS SHALL BE RECTIFIED AT THEIR COST
- 14. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE AND MAINTAIN THE INTEGRITY OF ALL SERVICES, CONDUITS AND PIPES DURING CONSTRUCTION. SPECIFICALLY DURING THE BACKFILLING AND COMPACTION PROCEDURE. ANY AND ALL DAMAGE TO NEW OR EXISTING SERVICES AS A RESULT OF THESE WORKS SHALL BE REPAIRED BY THE CONTRACTOR AT NO EXTRA COST.
- 15. PROTECT FINAL SURFACE WITH EITHER A TEMPORARY LOOSE SOIL LAYER OR A GRANULAR SUB-BASE LAYER TO PREVENT DRYING OUT PRIOR TO ON-GROUND SLAB CONSTRUCTION.
- 16. TESTING OF THE SUBGRADE SHALL BE CARRIED OUT BY AN APPROVED NATA REGISTERED LABORATORY AT THE CONTRACTORS EXPENSE.

ALL SETOUT TO ARCHITECT'S DRAWIN

DIMENSIONS TO BE VERIFIED WITH THE

ARCHITECT OR COMMENCING WORK.

NORTHROP ACCEPTS NO RESPONSIBILIT

FOR THE USABILITY, COMPLETENESS OF

SCALE OF DRAWINGS TRANSFERRED

FLECTRONICALLY

SERVICE TRENCHES

- 17. BACK FILL ALL TRENCHES UNDER NEW ROADS, PAVEMENTS, PATH AND BUILDINGS WITH DGS40 SUB-BASE MATERIAL COMPACTED TO 98% SMDD TO SUBGRADE LEVEL (UNO).
- 18. SAWCUT EXISTING SURFACES PRIOR TO EXCAVATION. BACK FILL ALL TRENCHES UNDER EXISTING ROADS, PAVEMENTS AND PATHS WITH STABILISED SAND 5% CEMENT OR DGS40 MATERIAL (5% CEMENT) COMPACTED IN 200mm THICK LAYERS TO 95% MMDD, (TOP 150mm COMPACTED TO 98% MMDD TO UNDERSIDE OF PAVEMENT).
- 19. BACKFILL ALL TRENCHES NOT UNDER ROADS, PAVEMENTS, PATHS AND BUILDINGS WITH APPROVED EXCAVATED OR IMPORTED MATERIAL COMPACTED TO 95% SMDD.

ASPHALTIC CONCRETE

GENERAL

- 1. ALL ASPHALTIC CONCRETE (AC) WORK TO BE PREPARED AND CARRIED OUT IN ACCORDANCE WITH GOOD ASPHALTIC PAVING PRACTICE AS DESCRIBED IN AS2734-1994 "ASPHALT (HOT-MIXED) PAVING - GUIDE TO GOOD PRACTICE" AND CURRENT RMS SPECIFICATIONS.
- PAVEMENT PREPARATION
- 2. THE EXISTING SURFACE TO BE SEALED SHALL BE WITHIN +/- 2% OF THE OPTIMUM AND BROOMED BEFORE COMMENCEMENT OF WORK TO ENSURE COMPLETE REMOVAL OF ALL SUPERFICIAL FOREIGN MATTER.
- 3. PRIME ALL SURFACES TO BE SEALED. ALLOW PRIME TO SETTLE FOR A MINIMUM OF 3 DAYS BEFORE APPLYING TACK COAT AND ASPHALT.
- 4. SWEEP PRIMED SURFACES BEFORE APPLYING TACK COAT AND ASPHALT. 5. ALL DEPRESSIONS OR UNEVEN AREAS ARE TO BE TACK-COATED AND
- BROUGHT UP TO GENERAL LEVEL OF PAVEMENT WITH ASPHALTIC CONCRETE BEFORE LAYING OF MAIN COURSE.
- 6. ALL DEFECTS IN THE BASE COURSE INCLUDING CRACKS, SURFACE DEFORMATION AND THE LIKE SHALL BE REPAIRED AS DIRECTED BY THE SUPERINTENDENT PRIOR TO PLACEMENT OF TACK COAT AND/OR AC COURSES. PLACEMENT
- 7. ALL ASPHALT SHALL BE PLACED UTILISING APPROVED MECHANICAL PAVING MACHINES. DO NOT HAND PLACE ASPHALT WITHOUT PRIOR APPROVAL FROM ENGINEER.

JOINTS

- 8. THE NUMBER OF JOINTS BOTH LONGITUDINAL AND TRANSVERSE SHALL BE KEPT TO A MINIMUM.
- 9. THE DENSITY AND SURFACE FINISH AT JOINTS SHALL BE SIMILAR TO THOSE OF THE REMAINDER OF THE LAYER.

COMPACTION

- 10. ALL COMPACTION SHALL BE UNDERTAKEN USING SELF PROPELLED ROLLERS
- 11. INITIAL ROLLING SHALL BE COMPLETED BEFORE THE MIX TEMPERATURE FALLS BELOW 105°C USING A STEEL DRUM ROLLER HAVING A MINIMUM WEIGHT OF 8 TONNES AND A MAXIMUM UNIT LOAD ON THE REAR DRUM EQUIVALENT TO 55kN/m WIDTH OF DRUM.
- 12. SECONDARY ROLLING SHALL BE COMPLETED BEFORE THE MIX TEMPERATURE FALLS BELOW 80°C USING A PNEUMATIC TYRED ROLLER OF A MINIMUM TOTAL LOAD OF 1 TONNE ON EACH TYRE.
- 13. ROLLED SURFACES SHALL BE SMOOTH AND FREE OF UNDULATIONS. BONY AND UNEVEN SURFACES WILL BE REJECTED.
- 14. PROVIDE 2 No. MINIMUM COMPACTION TESTS.
- **FINISHED SURFACE PROPERTIES**
- 15. FINISHED SURFACES SHALL BE SMOOTH, DENSE AND TRUE TO SHAPE AND SHALL NOT VARY MORE THAN:
- 15.1. 3mm FROM THE SPECIFIED PLAN LEVEL AT ANY POINT.
- 15.2. 3mm FROM THE BOTTOM OF A 3m STRAIGHT EDGE LAID TRANSVERSELY. 15.3. 5mm FROM THE BOTTOM OF A 3m STRAIGHT EDGE LAID LONGITUDINALLY.
- 15.4. MINUS 0 TO PLUS 2mm ADJACENT TO OTHER ELEMENTS SUCH AS KERBS AND THE LIKE TO AVOID POOLING OF SURFACE WATER.
- 15.5. MINUS 0 FROM THE SPECIFIED THICKNESS.
- 16. DO NOT STORE PLANT EQUIPMENT OR TRAFFIC NEWLY LAID ASPHALTIC CONCRETE PAVEMENTS WITHOUT PRIOR APPROVAL FROM THE ENGINEER.
- 17. DO NO APPLY MARKING PAINTS UNTIL ASPHALT HAS CURED IN ACCORDANCE WITH PAINT MANUFACTURER'S SPECIFICATIONS.

PAVEMENTS

- 1. ALL PAVEMENT MATERIALS SHALL COMPLY WITH CURRENT RMS SPECIFICATIONS. PROVIDE MECHANICAL ANALYSIS FOR EACH BATCH OF PAVEMENT MATERIAL TO ENSURE CONFORMITY.
- 2. COMPACTION STANDARDS: A) BASE 98% MODIFIED MAXIMUM DRY DENSITY (MMDD)
- B) SUB-BASE 95% MMDD
- C) SUBGRADE 100% STANDARD MAXIMUM DRY DENSITY (SMDD)
- 3. THE CONTRACTOR SHALL CONFIRM THE DESIGN CBR WITH A MINIMUM OF 3 TESTS TAKEN AT SUBGRADE LEVEL. WHERE DISCREPANCY IS FOUND, CONTACT THE DESIGNING ENGINEER.
- 4. ALLOW FOR COMPACTION TESTING BY NATA REGISTERED LABORATORY FOR: BASE LAYER, SUB-BASE LAYER, SUBGRADE IN ACCORDANCE WITH THE LATEST VERSION OF AS3798 FOR PAVEMENTS, (MINIMUM 2 TESTS PER LAYER). ALLOW FOR AT LEAST TWO SUCCESSFUL COMPACTION TESTS IN EACH LAYER.

STORMWATER DRAINAGE

- PIPES GREATER THAN OR EQUAL TO Ø300mm ARE TO BE MINIMUM CLASS 2 RUBBER-RING JOINTED RCP (UNO).
- 2. FRC PIPES EQUIVALENT TO THE STEEL REINFORCED CONCRETE PIPE CLASS SPECIFIED ON THE DRAWINGS MAY BE USED - OBTAIN SUPERINTENDENTS APPROVAL.
- 3. ALL PIPES ARE TO BE LAID AT (min) 1.0% GRADE (UNO).
- 4. U.N.O. THE USE OF PRE-CAST STORMWATER DRAINAGE PITS IS NOT ACCEPTED WITHOUT CONFIRMATION WITH NORTHROP CONSULTING ENGINEERS. THIS IS DUE TO THE GENERAL POOR QUALITY FINISH AND DAMAGE WHEN USED. THE CONTRACTOR AND NE ARE TO DISCUSS QUALITY CONTROL, AND CERTIFICATION OF FINISHES IF PRE-CAST PITS ARE PROPOSED.

PROJECT

RELEVANT AUSTRALIAN AND COUNCIL STANDARDS.

5. <u>COVERS</u>

NORTHROP

Bowral Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 2576 Ph (02) 4861 2042 P.O. Box 500. Bowral NSW 2576

Email bowral@northrop.com.au ABN 81 094 433 100

DRAWING TITLE

NOTES

A) USE HOT DIPPED GALVANISED COVERS AND GRATES COMPLYING WITH

SUB BASE PLATYPUS

B) ALL COVERS AND GRATES TO BE POSITIONED IN A FRAME AND MANUFACTURE AS A UNIT.

C) ALL COVERS AND GRATES TO BE FITTED WITH POSITIVE COVER LIFTING KEYS.

D) OBTAIN SUPERINTENDENT'S APPROVAL FOR THE USE OF CAST IRON SOLID COVERS AND GRATES. CAST IRON SOLID COVERS (IF APPROVED) TO CONSIST OF CROSS-WEBBED, CELLULAR CONSTRUCTION WITH THE RIBS UPPERMOST TO ALLOW INFILLING WITH CONCRETE. INSTALL POSITIVE COVER LIFTING KEYS AND PLASTIC PLUGS.

E) UNLESS DETAILED OR SPECIFIED OTHERWISE COVERS AND GRATES TO BE CLASS "C" IN VEHICULAR PAVEMENTS AND CLASS "B" ELSEWHERE.

- 6. ALL PIPE BENDS, JUNCTIONS, ETC. ARE TO BE PROVIDED USING PURPOSE MADE FITTINGS OR STORMWATER PITS. 7. ALL CONNECTIONS TO EXISTING DRAINAGE PITS SHALL BE MADE IN A
- TRADESMAN-LIKE MANNER AND THE INTERNAL WALL OF THE PIT AT PIPE PENETRATIONS SHALL BE CEMENT RENDERED TO ENSURE A SMOOTH FINISH 8. THE CONTRACTOR SHALL SUPPLY AND INSTALL ALL FITTINGS AND SPECIALS INCLUDING VARIOUS PIPE ADAPTERS TO ENSURE PROPER CONNECTION BETWEEN DISSIMILAR PIPEWORK.
- 9. U.N.O. MATERIAL USED FOR BEDDING OF PIPES SHALL BE APPROVED NON-COHESIVE GRANULAR MATERIAL HAVING HIGH PERMEABILITY AND HIGH STABILITY WHEN SATURATED AND FREE OF ORGANIC AND CLAY MATERIAL.
- 10. WHERE TRENCHES ARE IN ROCK, THE PIPE SHALL BE BEDDED ON A MIN. 50mm CONCRETE BED (OR 75mm THICK BED OF 12mm BLUE METAL) UNDER THE BARREL OF THE PIPE. THE PIPE COLLAR AT NO POINT SHALL BEAR ON THE ROCK
- 11. BEDDING SHALL BE (UNO) TYPE HS2 UNDER ROADS; H2 GENERAL AREAS, IN ACCORDANCE WITH CURRENT RELEVANT INDUSTRY STANDARDS AND GUIDELINES.
- 12. THE CONTRACTOR SHALL ENSURE AND PROTECT THE INTEGRITY OF ALL STORMWATER PIPES DURING CONSTRUCTION, ANY AND ALL DAMAGE TO THESE PIPES AS A RESULT OF THESE WORKS SHALL BE REPAIRED BY THE CONTRACTOR UNDER THE DIRECTION OF THE SUPERINTENDENT, AND AT NO EXTRA COST
- 13. NOTE THAT THE PIT COVER LEVEL NOMINATED IN GUTTERS ARE TO THE INVERT OF THE GUTTER WHICH ARE 40mm LOWER THAN THE PAVEMENT LEVEL AT LIP OF GUTTER.
- 14. PROVIDE 3.0m LENGTH OF Ø100 SUBSOIL DRAINAGE PIPE WRAPPED IN A NON-WOVEN GEOTEXTILE FABRIC, TO THE UPSTREAM SIDE OF STORMWATER PITS, LAID IN STORMWATER PIPE TRENCHES AND CONNECTED TO THE DRAINAGE PIT
- 15. Ø100mm SUB-SOIL DRAINAGE LINES WITH NON-WOVEN GEOTEXTILE SOCK SURROUND SHALL BE CONNECTED TO A STORMWATER DRAINAGE PIT (AT min. 1% LONGITUDINAL GRADE) AND PROVIDED IN THE FOLLOWING LOCATION: A) THE HIGH SIDE OF PROPOSED TRAFFICKED AND CARPARK PAVEMENT AREAS
- B) ALL PLANTER AND TREE BEDS PROPOSED ADJACENT TO PAVEMENT AREAS.
- C) BEHIND RETAINING WALLS (IN ACCORDANCE WITH DRAWINGS)
- D) ALL OTHER AREAS SHOWN ON THE DRAWINGS.
- 16. WHERE SUBSOIL DRAINAGE LINES PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS SEALED uPVC SEWER GRADE PIPE SHALL BE USED.

CONSTRUCTION HOLD POINTS

THE FOLLOWING ACTIONS CONSTITUTE A HOLD POINT. APPROVAL FROM THE DESIGN CIVIL ENGINEER/COUNCIL'S DEVELOPMENT ENGINEER IS REQUIRED PRIOR TO RELEASING THE HOLD POINT:

- HP1 PRIOR TO COMMENCEMENT OF THE WORKS IN ORDER TO VERIFY THE EXTENT OF WORKS.
- HP2 STORMWATER DRAINAGE WORKS PRIOR TO BACKFILLING/CONCRETE ENCASING.
- HP3 COMPACTION AND CBR TEST RESULTS FOR THE SUBGRADE.
- HP4 FORMWORK, REINFORCEMENT MESH AND JOINTS PRIOR TO ANY CONCRETE POUR.

NOTE THAT THE DESIGN CIVIL ENGINEER/COUNCIL'S DEVELOPMENT ENGINEER REQUIRES AT LEAST 48 HOURS PRIOR NOTICE THAT AN INSPECTION IS REQUIRED.

AS BUILT JOB NUMBER CONSTRUCTION SPECIFICATION 230309 DRAWING NUMBER REVISIO DRAWING SHEET SIZE = A1



					TABLE '1	' - PIT SC	HEDULE	STORMW	VATER	NETWOR	K 1	
PIT I.D.	PIT LID LEVEL	OUTLET DIAMETER	OUTLET INVERT RL	INLET DIAMETER	INLET INVERT RL	DEPTH	PIT SIZE	LID LOAD CLASS	STEP IRONS	EASTING	NORTHING	COMMENT
	(m)	(mm)	(m)	(mm)	(m)	(m)	(mm)			(m)	(m)	
EX1	7.073			150	6.470	0.603	-	-	-	335079.545	6253761.091	EXISTING PIT TO REMAIN
				225	7.272							
1\1	7.852	150	7.252	150	7.272	0.600	450x450	В	Ν	335080.795	6253751.894	PROPRIETARY SURFACE INLET PIT
				100 7	7.400							
2\1	8.174	225	7.574	150	7.594	0.600	450x450	В	N	335089.041	6253753.727	PROPRIETARY SURFACE INLET PIT
2\1	0.740	150	8 202	150	8.313	0.450	450,450	D	N	225006 270	6050740 444	
3\1	8.743	150	8.293	90	8.344	0.450	450x450	В	IN	335096.370	0253742.141	PROPRIETARY SURFACE INLET PIT
4\1	9.568	150	9.118	150	9.138	0.450	450x450	В	N	335103.144	6253729.876	PROPRIETARY SURFACE INLET PIT
5\1	10.013	150	9.563	150	9.583	0.450	450x450	В	Ν	335103.132	6253722.656	PROPRIETARY SURFACE INLET PIT
6\1	11.200	150	10.450	-	-	0.750	600x600	В	N	335090.715	6253719.096	PROPRIETARY SURFACE INLET PIT
VBD1/2	8.872	150	7.570	150	8.310	1.302	150	В	-	335081.212	6253749.536	PROPOSED VERTICAL BRANCH DRAIN, REFER TO DETAIL 'J'.
1\2	9.699	150	8.735	100	9.249	0.964	450x450	В	Ν	335082.135	6253747.623	PROPRIETARY SURFACE INLET PIT WITH VERTICAL BRANCH DRAIN, REFER TO DETAIL 'J'
2\2	10.996	100	10.586	-	-	0.410	450x450	В	N	335075.576	6253737.802	PROPRIETARY SURFACE INLET PIT



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04	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23		-
05	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	16/10/23		Turf Design Stud
В	AMMENDED GTD LENGTHS AND LEADER LOCATIONS	E.B.	M.B.	D.H.	C.N.	19/10/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)839
С	REVISIONS AS CLOUDED	E.B.	M.B.	D.H.	C.N.	15/11/23	P +61 8736 9000 W taylorau.com.au	
D	AS BUILT	E.B.	W.D.	D.H.		15/03/24	VERIFICATION SIGNATURE HAS BEEN ADDED	NORTHR









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01	ISSUED FOR 80% CO-ORDINATION	E.B.	M.B.	B.W.S.	C.N.	28/07/23		
02	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Stud
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	19/10/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)839
В	REVISIONS AS CLOUDED	E.B.	M.B.	D.H.	C.N.	15/11/23	P +61 8736 9000 W taylorau.com.au	Email: sydney@tt
С	AS BUILT	E.B.	W.D.	D.H.		15/03/24	VERIFICATION SIGNATURE HAS BEEN ADDED	NORTHR

LONGITUNDINAL SECTION FOR DRAINAGE LINE 1



2\2

(VBD1/2)

1\2

(1\1)

LONGITUNDINAL SECTION FOR DRAINAGE LINE 2



SUB BASE PLA

		AS BU	ILT	ed By :
	DRAWING TITLE	JOB NUMBER		Plott
ATYPUS	STORMWATER PIPE LONGITUDINAL	230309	ð	6am
	SECTION SHEET I	DRAWING NUMBER	REVISION	8:5
		C2.11	С	e : 28-3-24
		DRAWING SHEET	SIZE = A1	Date

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		.P. 8.76	.P. 8.66	.P. 8.37			P. 8.08	
		_	_	_			_	
			< <u>-11.36%</u> >	<	<	-5.93%	>	<-5.97%
DATUM R.L. 3.000								
HORIZONTAL						L=14.419		
GEOMETRY						B=00 27 23		
CUT/ FILL	+0.000	+0.000	+0.000	000.0+			+0.160	
DESIGN LEVELS	8.815	8.765	8.668	8.378			8.082	
EXISTING LEVELS	8.959 8.815	8.765	8.668	8.378			7.922	
CHAINAGE	0.000	0.944	1.800	5.000			10.000	





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01	ISSUED FOR 80% CO-ORDINATION	E.B.	M.B.	B.W.S.	C.N.	28/07/23		-
02	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Stu
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	15/11/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)839
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	DRAWING TITLE	JOB NUMBER		Plott
ATYPUS	MC01 LONGITUDINAL SECTION	230309	9	6am
	SHEET FOF I	DRAWING NUMBER	REVISION	8:5(
		C2.21	В	e : 28-3-24
		DRAWING SHEET	SIZE = A1	Date



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NSW 2008 Phone (+61 2)8394 9990 Email: sydney@turfdesign.com	SCALE 1:40@	A1 L	0.4 I	0.8	1.2 I	1.6 2.0n	ſ	Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 2576 Ph (02) 4861 2042		
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		AS BU	ILT	ed By :
	DRAWING TITLE	JOB NUMBER		Plotte
TYPUS	MC01 CROSS SECTIONS SHEET 1	230309	9	6am
		DRAWING NUMBER	REVISION	8:5(
		C2.31	В	: 28-3-24
		DRAWING SHEET	SIZE = A1	Date

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RL6.500m RL6.500m DESIGN CODE CUT/FILL DESIGN LEVELS EXISTING EXISTING DESIGN LEVELS DESIGN OFFSET Horizonal:-40.00 Vertical : - 40.00	DESIGN LEVELS (TYPICAL).	1 84.6 8.430 8.401 -0.082 2 141 8.558 8.870 0.315 2 141 8.556 8.870 0.315 2 752 8.656 8.742 0.086 2 7.13 8.556 8.742 0.086 3 110 8.742 0.086 0.14% 3 132 8.711 8.900 0.139 3 110 8.61 9.01 0.199 3 132 8.711 8.900 0.14% 3 132 8.711 8.900 0.189 3 132 8.711 8.900 0.14% 3 740 8.811 9.601 0.901 3 794 8.811 0.607 0.608 4 266 8.813 0.606 0.901 4 266 8.811 0.901 0.901 4 9.911 0.9466 0.901 0.901 4 9.901 0.9466 0.901 0.901 9.	9.86% 10.76% INDICATIVE DEPTH BULK EARTHWORK LAYER (TYPICAL). INDICATIVE DEPTH BULK EARTHWORK LAYER (TYPICAL). INDICATIVE DEPTH VIRGIN SANDSTON INDICATIVE DE	1 OF I OF RL6.500m RL6.500m DESIGN CODE CUT/FILL DESIGN DESIGN CUT/FILL DESIGN DESIGN CUT/FILL DESIGN DESIGN DESIGN CUT/FILL DESIGN DESIGN DESIGN POFSET	A3:53% 2.37% 6.40% 9 A3:53% 2.37% 6.40% 9 A4:53% 2.37\% 2	66% 6.70% 36% 66% 6.70% 036% 11 11 11 12 11 11 13 11 11 14 11 11 15 11 11 16 0 0 10 0 0 11 11 11 12 11 11 13 12 11 14 14 11 15 14 11 16 0 0 17 14 11 18 8 8 19 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10
EV.DESCRIPTION1ISSUED FOR 80% CO-ORDINATION2ISSUED FOR 100% CO-ORDINATION3ISSUED FOR 100% CO-ORDINATIONAISSUED FOR CONSTRUCTION3AS BUILT	DESIGN DRAWN CHECKED VERIFIED DATE E.B. M.B. B.W.S. C.N. 28/07/23 E.B. M.B. D.H. C.N. 09/08/23 E.B. J.W.S. D.H. C.N. 28/09/23 E.B. M.B. D.H. C.N. 28/09/23 E.B. M.B. D.H. C.N. 15/11/23 E.B. W.D. D.H. 15/03/24	CLIENT Level 16, 100 Pacific Highway North Sydney NSW 2060 P +61 8736 9000 W taylorau.com.au DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION SIGNATURE HAS BEEN ADDED	LANDSCAPE ARCHITECT Turf Design Studio : P.O. Box 2282 Sydney NSW 2001, 35 Wellington St Chippendal NSW 2008 Phone (+61 2)8394 9990 Email: sydney@turfdesign.com THE COPYRIGHT OF THIS DRAWING REMAINS WITH NORTHROP CONSULTING ENGINEERS PT LTD.	ALL SETOUT TO ARCHITECT'S DRAWINGS, DIMENSIONS TO BE VERIFIED WITH THE ARCHITECT OR COMMENCING WORK. NORTHROP ACCEPTS NO RESPONSIBILITY FOR THE USABILITY, COMPLETENESS OR SCALE OF DRAWINGS TRANSFERRED ELECTRONICALLY. 0 0.4 0.8 1.2 1.6 2.0m SCALE 1:40 @A1	A Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 257 Ph (02) 4861 2042 P.O. Box 500, Bowral NSW 2576 Email bowral@northrop.com.au ABN 81 094 433 100	PROJECT SUB BASE PLATYP

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	DRAWING TITLE	JOB NUMBER	
YPUS	MC01 CROSS SECTIONS SHEET 2	230309	•
		DRAWING NUMBER	REVISION
		C2.32	В
		DRAWING SHEET	SIZE = A1







REV	DESCRIPTION	DESIGN	IDRAWN	CHECKED	VERIFIED	DATE	CLIENT	LANDSC
01	ISSUED FOR 80% CO-ORDINATION	E.B.	M.B.	B.W.S.	C.N.	28/07/23		- -
02	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Stu
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	15/11/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)83
В	AS BUILT	E.B.	W.D.	D.H.		15/03/24	P +61 8736 9000 W taylorau.com.au	Email: sydney@i
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15.000	060.01	cc/.01	062.11	11.300	694.0+
18.000	10.050	10.669	11.250	11.300	+0.705
20.000	9.850 9.850	10.612	11.250	11.300	+0.638
25.000	9.850 9.850	10.539	11.250	11.300	+0.711
26.286	9.850	10.520	11.250	11.300	+0.730
26.325	9.850	10.520	11.250	11.300	+0.730
30.000	9.850	10.405	11.250	11.300	+0.845
31.291	9.850	10.365	11.250	11.300	+0.885
35 000	0.850	062 01	11 250	11 300	0960+
37.474	9.850	10.255	11.250	11.300	+0.995
37.691	9.050	9.241	11.250	11.300	+2.009
39.300	9.050	9.199 10.610	11.250	11.300	+2.051
39.829	10.450	10.630	11.250	11.300	+0.620

LONGITUDINAL SECTION ALONG CONTROL LINE RW02

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34 9990 Irfdesign.com	SCALE 1:100	@A1	1 2 I I	3 	4 5m	Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 2576 Ph (02) 4861 2042		
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EXISTING UTILITY SERVICES ARE BASED ON DIAL BEFORE YOU DIG AND LAND SURVEY INFORMATION. THIS INFORMATION DOES NOT PROVIDE DEPTHS, ACCURATE ALIGNMENTS OR SIZES. NORTHROP HAS NOT CHECKED FOR CLASHES WITH PROPOSED WORKS NOR DO WE CONFIRM SERVICES SHOWN ARE ACCURATE OR REPRESENT EXISTING. NORTHROP RECOMMENDS ADDITIONAL INVESTIGATIONS ARE UNDERTAKEN WHICH INCLUDES ELECTRONIC TRACING AND POT HOLING.





REV	2. DESCRIPTION	DESIGN	DRAWN	CHECKED	VERIFIED	DATE	CLIENT	LANDSCAPE ARCHITECT	ALL SETOUT TO ARCHITECT'S DRAWINGS, DIMENSIONS TO BE VERIEVED WITH THE		PROJECT
01	ISSUED FOR 80% CO-ORDINATION	E.B.	M.B.	B.W.S.	C.N.	28/07/23			ARCHITECT OR COMMENCING WORK.		SUB BASE PLA
02	ISSUED FOR 100% CO-ORDINATION	E.B.	M.B.	D.H.	C.N.	09/08/23			FOR THE USABILITY, COMPLETENESS OR		
03	ISSUED FOR 100% CO-ORDINATION	E.B.	J.W.S.	D.H.	C.N.	28/09/23		Turf Design Studio : P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippendale,	SCALE OF DRAWINGS TRANSFERRED FLECTRONICALLY	Bowral	
А	ISSUED FOR CONSTRUCTION	E.B.	M.B.	D.H.	C.N.	19/10/23	Level 16, 100 Pacific Highway North Sydney NSW 2060	NSW 2008 Phone (+61 2)8394 9990		Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 2576	
В	REVISIONS AS CLOUDED	E.B.	M.B.	C.N.	C.N.	15/11/23	P +61 8/36 9000 W taylorau.com.au		AS SHOWN	Ph (02) 4861 2042 P.O. Box 500, Bowral NSW 2576	
С	AS BUILT	E.B.	W.D.	D.H.		28/03/24	VERIFICATION SIGNATURE HAS BEEN ADDED	NORTHROP CONSULTING ENGINEERS PTY LTD.		Email bowral@northrop.com.au ABN 81 094 433 100	

FLECTRONICALLY

AS SHOWN

Bowral

Shop 9A 'High Street Arcade' 310-312 Bong Bong Street, Bowral NSW 2576

Ph (02) 4861 2042

P.O. Box 500, Bowral NSW 2576

Email bowral@northrop.com.au ABN 81 094 433 100

Turf Design Studio : P.O Box 2282 Sydney NSW 2001, 35 Wellington St Chippenda

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NSW 2008

Phone (+61 2)8394 9990 Email: sydney@turfdesign.com

A ISSUED FOR CONSTRUCTION

B REVISIONS AS CLOUDED

C AS BUILT

E.B. M.B. D.H.

E.B. M.B. D.H.

E.B. W.D. D.H.

C.N. 19/10/23

C.N. 15/11/23

28/03/24

Level 16, 100 Pacific Highway North Sydney NSW 2060

DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS

VERIFICATION SIGNATURE HAS BEEN ADDED

P +61 8736 9000 W tavlorau.com.au

- 1. TRENCH WIDTH MAY NEED TO INCREASE SUBJECT TO ACHIEVING ADEQUATE
- - BACKFILL COMPACTED DGB20 TO 98% SMDD IF IN TRAFFICABLE

PIPE OVERLAY - COMPACTED SELECT FILL (NOMINAL 10mm AGGREGATE) TO SUBGRADE LEVEL IF UNDER ROAD OR UP TO BACKFILL LEVEL IN NON TRAFFICABLE AREAS.
PIPE SIDE SUPPORT - COMPACTED SELECT FILL (NOMINAL 10mm AGGREGATE)
BEDDING - SELECT FILL (NOMINAL 10mm AGGREGATE)

TION	MINIMUM H* m
E LOADING	0.30
LOADING -	
3	0.45
AYS	0.60
ROADWAYS	0.75
OR SUBJECT TO ENT LOADS	0.75

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	TABLE '3' - CUT AND FILL VOLUMES								
STAGE	CUT (SANDSTONE)	CUT (TOTAL)	FILL (TOTAL)	ESTIMATED CUT FOR HORIZON 'B TREE ROOT BALL'	ESTIMATED CUT FOR PROPOSED STRUCTURAL ELEMENTS	ESTIMATED CUT FOR ELECTRICAL AND TELECOMMUNICATIONS TRENCH EXCL. BACKFIL ⁵	ESTIMATED CUT FOR STORMWATER TRENCH EXCL. BACKFIL ⁵	NET	
TOTAL	18 cu.m	339 cu.m	233 cu.m	92cu.m	71cu.m	58 cu.m	26 cu.m	353 cu.m (CUT)	

REV.	DESCRIPTION	DESIGN	DRAWN	CHECKED	VERIFIED	DATE	CLIENT	LANDSC
А	ISSUED FOR CONSTRUCTION	E.B.	J.W.S.	D.H.	C.N.	07/09/23		-
В	AMMENDEMNT TO TABLE '1' AND NOTES	E.B.	W.D.	D.H.		15/11/23		
С	AS BUILT	E.B.	W.D.	D.H.		15/03/24		Turf Design Stud
							Level 16, 100 Pacific Highway North Sydney NSW 2060	Phone (+61 2)839
							P +61 8/36 9000 W taylorau.com.au	Email: sydney@tu
							DRAWING NOT TO BE USED FOR CONSTRUCTION UNLESS	THE COPY
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s.com.au	SHT. SIZE DRG	E020-AS-SIS-G	S		^{REV} AB	

SUBBASE PLATYPUS

120 High St, North Sydney NSW 2060 **STRUCTURAL DOCUMENTATION**

			-				
REV	DESCRIPTION	ISSUED	VER'D	APP'D	DATE	CLIENT	ARCHITECT
1	PRELIMINARY ISSUE	HA		RP	11.05.23		
2	ISSUED FOR TENDER	HA		RP	24.05.23		
3	REVISED TENDER ISSUE	HA		RP	10.08.23	IAILUK	
4	REVISED TENDER ISSUE	NTH		RP	08.09.23		
5	AS BUILT	TAT		RP	21.02.24		
						DRAWINGS NUT TO BE USED FOR LONSTRUCTION UNLESS	
						VERIFICATION SIGNATURE HAS BEEN ADDED	CON

Wollongong Level 1, 57 Kembla Street, Wollongong, N.S.W. 2500 Ph (02) 4226 3333 Email: southcoast@northrop.com.au ABN 81 094 433 100

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GENERAL

- G1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH SPECIFICATIONS AND OTHER CONSULTANT'S DRAWINGS
- **G2.** THE WEATHER PROOFING OF THE BUILDING IS THE BUILDER'S RESPONSIBILITY. THIS INCLUDES (BUT IS NOT LIMITED TO) THE SPECIFICATION AND FIXING DETAILS OF CLADDINGS, SHEETING, FLASHING, MEMBRANES, STEPS, SETDOWNS & RECESSES.
- G3. ALL DISCREPANCIES SHALL BE REFERRED TO THE PROJECT MANAGER AND RESOLVED BEFORE PROCEEDING WITH THE WORK.
- G4. ALL DIMENSIONS SHOWN SHALL BE VERIFIED BY THE BUILDER ON SITE. THESE STRUCTURAL DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS. THE RL'S SHOWN ON THESE DRAWINGS ARE APPROXIMATE AND ARE FOR THE SOLE PURPOSE OF ASSISTING THE STRUCTURAL DOCUMENTATION. THEY ARE NOT TO BE USED FOR CONSTRUCTION PURPOSES. REFER TO ARCHITECTURAL DRAWINGS FOR CONFIRMATION OF ALL RL'S, ALL LEVELS ARE IN METRES (m) AND DIMENSIONS ARE IN MILLIMETRES (mm)
- G5. ALL WORKMANSHIP, TESTING, MATERIALS AND SUPERVISION ARE TO BE IN ACCORDANCE WITH THESE SPECIFICATIONS, THE WORK HEALTH AND SAFETY ACT 2011. ENFORCED BY THE WORKCOVER AUTHORITY AND CURRENT RELEVANT AUSTRALIAN STANDARDS.
- G6. PROPRIETARY ITEMS SPECIFIED SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN RECOMMENDATIONS. DO NOT VARY SPECIFIED PROPRIETARY PRODUCTS WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
- G7. THESE DRAWINGS AND ISSUED WRITTEN INSTRUCTIONS DURING THE COURSE OF THE CONTRACT DEPICT THE COMPLETE STRUCTURE. THEY DO NOT DESCRIBE A WORK METHOD. THE ARRANGEMENT. DESIGN AND INSTALLATION OF TEMPORARY WORKS REMAINS THE RESPONSIBILITY OF THE CONTRACTOR.
- **G8.** THE DETERMINATION OF A SAFE WORK METHOD REMAINS THE RESPONSIBILITY OF THE CONTRACTOR. ANY ELEMENT WHICH POSES AN UNACCEPTABLE LEVEL OF SAFETY RISK TO CONSTRUCT SHALL BE REFERRED TO THE STRUCTURAL ENGINEER. TEMPORARY BRACING AND SUPPORT OF STRUCTURE IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MAINTAINED DURING ALL STAGES OF CONSTRUCTION.
- **G9.** NOTES ON ANY DRAWING APPLY TO ALL DRAWINGS IN THE SET UNLESS NOTED OTHERWISE
- G10. ALL ARCHITECTURAL FITMENTS SUCH AS GLAZING, PARTITIONS, CEILINGS ETC. SHOULD ALLOW FOR THE SHORT AND LONG TERM MOVEMENT OF STRUCTURAL ELEMENTS. FOR BEAMS AND SLABS SPANNING LESS THAN 8m AN ALLOWANCE OF AT LEAST 20mm SHOULD BE MADE (CONSULT ENGINEER WHERE SPANS EXCEED 8m).
- G11. THE BUILDER SHALL PROVIDE CERTIFICATION ON ANY DESIGN AND CONSTRUCT COMPONENT BY A CHARTERED PROFESSIONAL ENGINEER (NER).
- G12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION OF ALL SERVICES IN THE VICINITY OF THE WORKS. ANY SERVICES SHOWN ARE PROVIDED FOR INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM THE LOCATION OF ALL SERVICES PRIOR TO COMMENCING AND SHALL BE RESPONSIBLE FOR THE REPAIR OF ANY DAMAGE CAUSED TO SERVICES, AS WELL AS ANY LOSS INCURRED AS A RESULT OF THE DAMAGE TO ANY SERVICE.
- G13. THE STRUCTURAL COMPONENTS DETAILED ON THESE STRUCTURAL DRAWINGS ARE JOB SPECIFIC AND HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RELEVANT AUSTRALIAN STANDARDS AND BUILDING CODE OF AUSTRALIA FOR THE FOLLOWING LOADS.

	WIND LOADS:	
	- IMPORTANCE LEVEL	= 2
	– REGION	= A2
	- ANNUAL PROBABILITY OF EXCEDENCE	= 1 : 500
	- REGIONAL WIND SPEED V	= 45 m/s
	- TERRAIN CATEGORY	= TC2
	– TERRAIN MULTIPLIER Mz,cat	= 1.0
	- WIND DIRECTION MULTIPLIER Md	= 1.0
	- SHIELDING MULTIPLIER Ms	= 1.0
	- TOPOGRAPHIC MULTIPLIER Mt	= 1.0
	- SITE WIND SPEED	= 45 m/s
	LIVE LOADS:	
	- PAVEMENTS	= 5 kPa
	- STAIRS	= 5 kPa
	EARTHQUAKE LOADS:	
	- IMPORTANCE LEVEL	= 2
	 ANNUAL PROBABILITY OF EXCEDENCE (P) 	= 1:500
	– PROBABILITY FACTOR (kp)	= 1.0
	- HAZARD FACTOR (Z)	= 0.08
	- DOMESTIC STRUCTURE?	= N
	- SITE SUB-SOIL CLASS	= Be – SHALLOW SOIL
	 EARTHQUAKE DESIGN CATEGORY (EDC) 	=
	- DESIGN REQUIRED	= SIMPLE STATIC CHECH
514 .	ANY PRODUCTS SPECIFIED OR USED ARE TO BE VERIFIED BY THE CONTR	ACTOR AS BEING SAFE AND

ANY PRUDULIS SPELIFIED OR USED ARE TO BE VERIFIED BY THE LUNTRALIOR AS BEING SAFE AND APPROPRIATE FOR USE. NORTHROP CONSULTING ENGINEERS DO NOT TAKE ANY RESPONSIBILTY FOR THE USE OF UNSAFE PRODUCTS

G15. DRAWING TO BE READ IN CONJUNCTION WITH THE REQUIREMENTS OF THE REMEDIATION ACTION PLAN PREPARED BY CONSARA NUMBERED C201/015_DFTRAP_110CT2022 AND DATED 15 NOVEMBER 2022.

CONSTRUCTION PHASE SERVICES (WITNESS POINTS)

- WP1. OBTAIN NORTHROP CONSULTING ENGINEERS WRITTEN INSTRUCTION AT THE FOLLOWING HOLD POINTS:
- PREPARATION OF FOUNDING MATERIAL, INCLUDING PIER BORE HOLES.
- REINFORCEMENT PRIOR TO PLACEMENT OF CONCRETE or COREFILLING OF BLOCKWORK WP2. PROVIDE MINIMUM 48 HOURS NOTICE FOR ANY REQUIRED INSPECTIONS.

TEMPORARY WORKS

TW1. THESE DRAWINGS DEPICT THE "PERMANENT" STRUCTURE, TEMPORARY WORKS REMAIN THE RESPONSIBILITY OF THE CONTRACTOR.

- TW2. BUILDER MUST ENGAGE (NER) QUALIFIED STRUCTURAL ENGINEER FOR THE DESIGN OF ALL TEMPORARY WORKS NECESSARY TO SAFELY ERECT THIS STRUCTURE. AS A MINIMUM THE FOLLOWING WORKS REQUIRE ATTENTION;
- FORMWORK / TEMPORARY PROPPING / NEEDLE BEAMS / SCAFFOLDING / UNDERPINNING / TEMPORARY SHORING TW3. BUILDER SHALL CONTACT NORTHROP CONSULTING ENGINEERS IF THEY CONSIDER ANY PART OF THIS STRUCTURE IS UNSAFE TO ERECT

EARTHWORKS UNDER GROUND SLABS

- E1. REMOVE ALL TOPSOIL, ORGANIC MATERIAL AND FILL BENEATH BUILDING PLATFORM. ALSO EXCAVATE TO THIS DEPTH FOR A DISTANCE OF D + 0.5m AROUND THE PERIMETER OF THE BUILDING, WHERE D IS THE DEPTH OF EXCAVATION.
- E2. SOIL AND BUILDING MATERIAL REMOVED OFF SITE MUST BE DONE SO IN ACCORDANCE WITH THE NSW EPA GUIDELINES. **E3.** IMPORTED FILL PROPERTIES
 - PASSING 50mm SIEVE 100%
 - PASSING 75micron SIEVE LESS THAN 25% - PLASTICITY INDEX - LESS THAN 15% BUT MORE THAN 2%
- RFV DESCRIPTION ISSUED VER'D APP'D DATE CLIENT 11.05.23 PRELIMINARY ISSUE RP HA TAYLOR ISSUED FOR TENDER RP 24.05.2 HA RP 10.08.2 REVISED TENDER ISSUE HA NTH REVISED TENDER ISSUE RP 08.09.23 TAT AS BUILT RP 21.02.24 DRAWINGS NOT TO BE USED FOR CONSTRUCTION UNLESS VERIFICATION SIGNATURE HAS BEEN ADDED CONSULTING ENGINEERS PTY LTD

EARTHWORKS UNDER GROUND SLAB (CONTINUED)

- E4. ONLY IMPORTED MATERIAL CLASSIFIED AS "INERT" UNDER NSW EPA GUIDELINES TO BE USED. **E5.** FILL WITHIN BUILDING PLATFORMS SHOULD BE PLACED AND COMPACTED AS FOLLOWS:
 - IN LAYERS OF 200 MAXIMUM LOOSE THICKNESS.

 - LAYER TO PREVENT DRYING OUT PRIOR TO SLABCONSTRUCTION.
 - DEFINED IN SECTION 8 OF AS3798-2007) DUE TO THE TIGHT LIMITS RECOMMENDED ABOVE.

LIEU OF CRUSHED ROCK IN APPROPRIATE CIRCUMSTANCES.

- REPORT U.N.O.
- CHECK WITH THE RELEVANT ENGINEER.
- GROUND OR COMPACTED IMPORTED FILL.
- **TB4.** IMPORTED FILL PROPERTIES
- PASSING 50mm SIEVE 100% - PASSING 75micron SIEVE - LESS THAN 25%
- PLASTICITY INDEX LESS THAN 15% BUT MORE THAN 2%
- TB6. ALLOW FOR 1 SUCCESSFUL COMPACTION TEST PER 20 METRES LENGTH OF TRENCH IN THE MIDDLE LAYER.

FOUNDATIONS

- F1. ASSUMED ALLOWABLE BEARING CAPACITY:
 - PAD FOOTINGS
 - STRIP FOOTINGS / EDGE THICKENING = 80 kPa
 - SLABS ON GROUND - BORED PIERS (UNO)
- AT OR BELOW GROUND LEVEL. CLASSIFICATION.
- NORTHROP CONSULTING ENGINEERS PRIOR TO CONCRETING FOUNDATIONS.
- F5. ENSURE STABILITY OF ADJACENT BUILDINGS AND PATHS IS MAINTAINED DURING ALL STAGES OF CONSTRUCTION.
- - ALEIELE FOOTING ZONE OF INFLUENCE LINE TO BE DETERMINED BY
 - ENGINEER (ASSUME 45° FOR TENDER PURPOSES) -

BASE OF TRENCH OR TOP OF 10MPa CONCRETE BACKFILL TO TRENCH -

FOOTING ———	The second se

WHERE ADDITIONAL EXCAVATION IS REQUIRED DUE TO UNSATISFACTORY FOUNDATION MATERIAL, POUR 10MPa MASS CONCRETE TO UNDERSIDE OF FOOTING. -

- STRUCTURAL DRAWINGS
- DRYING OUT BY EXPOSURE.

SLAB ON GROUND

SG1. UNLESS NOTED OTHERWISE SLABS TO BE 100mm THICK WITH SL82 FABRIC THROUGHOUT TOP. CAST SLABS ON 0.2mm POLYTHENE MEMBRANE LAID OVER A NOMINAL SAND LEVELING LAYER ON FIRM CUT ORIGINAL GROUND OR COMPACTED FILL AS SPECIFIED.

SG2. MESH LAPS: TRANSVERSE WIRES OF SHEET BEING LAPPED, AS SHOWN BELOW.

•	•	•	• •	••
		•		50mr

THE STRUCTURAL ENGINEER PRIOR TO CONCRETE BEING PLACED.

- TO A DRY DENSITY RATIO OF: 98% STANDARD DRY DENSITY FOR COMMERCIAL BUILDINGS

- WITH A MOISTURE CONTENT BETWEEN 1% DRY & 2% WET OF OMC. - THE FINAL SURFACE COVERED WITH EITHER A TEMPORARY LOOSE SOIL LAYER OR A GRANULAR SUB-BASE

- EARTHWORKS TO BE CARRIED OUT WITH GEOTECHNICAL INSPECTION AT LEVEL 1 RESPONSIBILITY (AS

E6. IF SUBGRADE OR FILL IS TOO WET TO ACHIEVE COMPACTION. SCARIFY AND WORK MATERIAL UNTIL IT HAS DRIED. E7. NORTHROP CONSULTING ENGINEERS SUPPORT THE USE OF CONTAMINANT FREE, RECYCLED CONCRETE OR BRICK IN

TRENCH BACKFILL AND UNDERGROUND SERVICES

TB1. ALLOW FOR EXCAVATION IN ALL MATERIALS AS FOUND ON SITE AND AS DETAILED IN GEOTECHNICAL

TB2. PRIOR TO COMMENCING EXCAVATION VERIFY LEVELS OF ALL EXISTING SERVICES. IF ANY DISCREPANCIES

TB3. BACKFILL ALL TRENCHES UNDER SLABS, PATHS AND ROADS BELOW SUB-BASE LEVEL WITH ORIGINAL

TB5. COMPACT FILL TO 95% MAXIMUM MODIFIED DRY DENSITY EXCEPT LANDSCAPED AREAS WHICH SHALL BE 85% MAXIMUM MODIFIED DRY DENSITY. COMPACT IN LAYERS OF 200 mm MAXIMUM LOOSE THICKNESS.

- = 80 kPa = 80 kPa
 - = 1000 kPa END BEARING

F2. A GEOTECHNICAL REPORT HAS BEEN CARRIED OUT, REFER TO REPORT No. 7P2839 01 PREPARED BY MORROW GEOTECHNICS. THIS REPORT IS FOR INFORMATION ONLY, IT IS NOT A COMPLETE DESCRIPTION OF CONDITIONS

F3. THE SLAB AND FOOTINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH AS2870-2011 FOR CLASS H1 SITE. A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER TO BE CONTACTED DURING EXCAVATION TO CONFIRM THE SITE

F4. THE CONTRACTOR SHALL ALLOW TO ENGAGE A QUALIFIED (NER) GEOTECHNICAL ENGINEER TO APPROVE THE FOUNDATION MATERIAL. OBTAIN GEOTECHNICAL ENGINEERS APPROVAL AND SUBMIT CERTIFICATE IN WRITING TO

F6. DO NOT ALLOW EXCAVATED MATERIAL TO BE STOCKPILED WITHIN 1500mm OF FOOTING TRENCHES OR PITS. NO EARTH OR DETRITUS IS TO FALL INTO THE FOOTING TRENCHES BEFORE OR DURING CONCRETE PLACEMENT.

F7. THE UNDERSIDE OF FOUNDATIONS SHALL CONFORM TO THE FOLLOWING REGARDLESS OF NOMINATED LEVELS:

MATERIAL F8. FOOTINGS SHALL BE CENTRALLY LOCATED UNDER WALLS AND COLUMNS UNLESS NOTED OTHERWISE ON THE

F9. FOOTINGS SHALL BE EXCAVATED TO THE DETAILED DEPTH AND WIDTH. FOOTINGS SHALL BE INSPECTED AND FILLED WITH CONCRETE AS SOON AS POSSIBLE TO AVOID EITHER SOFTENING OF THE FOUNDATION MATERIAL OR

F10. THE BASE OF ALL PIER HOLES SHALL BE FREE OF WATER AND CLEANED OF LOOSE MATERIAL OR DEBRIS PRIOR TO PLACEMENT OF CONCRETE. ALLOW TO PROVIDE TEMPORARY LINERS AS DEEMED NECESSARY.

THE TWO OUTERMOST TRANSVERSE WIRES OF ONE SHEET OF MESH MUST OVERLAP THE TWO OUTERMOST

- • • • nm OVERLAP OF END WIRE 1

SG3. PROVIDE 3-N12 TRIMMER BARS 2000mm LONG TIED TO UNDERSIDE OF FABRIC AT ALL RE-ENTRANT CORNERS. SG4. ALL CONCRETE IS TO BE PLACED USING A VIBRATOR. CURING METHODOLOGY TO BE APPROVED IN WRITING BY

CONCRETE

C1. CARRY OUT ALL CONCRETE WORK IN ACCORDANCE WITH AS3600-2018 AND NATSPEC CONCRETE STANDARDS. C14. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY, AND NOT NECESSARILY IN TRUE PROJECTION. BARS **C2.** CONCRETE PROPERTIES AND COVER TO REINFORCING

COVER TO REINFORCEMENT							
	ELEMENT	CONCRETE STRENGTH f'c (MPa)	MAXIMUM 56 DAY DRY SHRINKAGE	COVEF	? (mm)		
BORED PIERS		40		60			
SLABS ON	SLABS ON EXTERNAL (NO MEMBRANE)		(50,00	TOP 45	BTM 40		
GROUND INTERNAL (WITH MEMBRANE)		40	ווום עכס	TOP 45	BTM 35		
STRIP FOOTINGS	INGS 32 60		0				
PAD FOOTINGS		32		6	0		
SUSPENDED SLABS	EXTERNAL	50	650 um	TOP 45	BTM 45		
	EXTERNAL FACE	50	(50)		45		
WALLS	IN CONTACT WITH GROUND	50	וווט עכס	45			

MAXIMUM AGGREGATE SIZE = 20mm U.N.O.

SLUMP DURING PLACING = 80mm ±10mm EXPOSURE CLASSIFICATION = B2 (EXTERNAL CONCRETE ELEMENTS)

NO ADMIXTURES SHALL BE USED IN THE CONCRETE MIX UNLESS APPROVED BY NORTHROP CONSULTING ENGINEERS IN WRITING.

- **C3.** CONCRETE PROPERTIES FOR SLABS AND BEAMS SHALL BE VARIED FROM NORMAL CLASS AS FOLLOWS: MINIMUM CEMENT CONTENT 250kg/m³
 - MAXIMUM 56 DAY SHRINKAGE STRAIN = AS NOMINATED ABOVE
 - PRIOR TO COMMENCEMENT CONCRETE SUPPLIER TO PROVIDE DRYING SHRINKAGE TEST RESULTS FROM PRODUCTION ASSESSMENT AS EVIDENCE THAT SPECIFIED DRYING SHRINKAGE LIMITS CAN BE ACHIEVED USING NORMAL MIX DESIGN.
 - PERCENTAGE OF ENTRAPPED AIR TO BE AS FOLLOWS:

FOR AGGREGATE 10mm-20mm NORMAL SIZE 8-4% IN ACCORDANCE WITH AS3600-2018 AND AS1012.4-2014 (SUBMIT TEST RESULTS) FOR ALPINE OR SUB-ALPINE AREAS.

- **C4.** SUBMIT FOR APPROVAL THE FOLLOWING TO THE ENGINEER
 - CURING PROCEDURE (PVA MEMBRANES NOT PERMITTED)
 - STRIPPING AND BACK PROPPING PROCEDURE
 - DETAILS AND LOCATION OF CONDUITS AND PENETRATIONS
 - CONSTRUCTION JOINT LOCATIONS
 - MINIMUM STRIPPING TIMES ARE IN AS3600 NOW.
- C5. FOR TENDER PURPOSES ASSUME MINIMUM STRIPPING TIMES AND EXTENT OF BACK PROPPING AS PER AS3600-2018 ANDAS3610.1-2018 AND AS PER GENERAL NOTES FOR FORMWORK AND PROPPING. C6. FORMWORK FINISH CLASSIFICATION TO AS3610.1-2018:

 - ELEMENT CLASS – INGROUNI

– INGROUND FOOTINGS	5
- RETAINING WALLS	5 EARTH FACE

- RETAINING WALLS	2 EXPOSED FACE
	2

- STAIRS **C7.** SURFACE FINISHES
 - COLUMNS & WALLS OFF FORM
 - FLOOR SLABS (U.N.O.) MACHINE FLOAT
 - SLABS TO BE TILED WOOD FLOAT
 - STAIRS STEEL TROWEL
- **C8.** COMPACT ALL CONCRETE, INCLUDING FOOTINGS AND SLABS USING MECHANICAL VIBRATORS. C9. PLACE CONCRETE CONTINUOUSLY BETWEEN CONSTRUCTION JOINTS SHOWN ON PLAN. DO NOT BREAK OR INTERRUPT SUCCESSIVE POURS SUCH THAT COLD JOINTS OCCUR. ANY REVISIONS OR ADDITIONS TO CONSTRUCTION JOINTS SHOWN ON PLAN REQUIRE APPROVAL FROM THE NORTHROP CONSULTING
 - ENGINEERS.
- **C10.** CONCRETE PROFILES
 - BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE THE SLAB THICKNESS.
 - SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.
 - NO HOLES, CHASES OR EMBEDMENT OF PIPES OTHER THAN SHOWN IN THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR WRITTEN APPROVAL OF
 - NORTHROP CONSULTING ENGINEERS. - PROVIDE DRIP GROOVES AT ALL EXPOSED EDGES. CHAMFERS, DRIP GROOVES, REGLETS ETC TO
- ARCHITECT'S DETAILS. C11. ALL PENETRATIONS TO HAVE 2/N16 TRIMMER BARS TOP AND BOTTOM TO EACH FACE. U.N.O. EXTEND
- TRIMMERS 600 BEYOND PENETRATION. C12. SETDOWNS OR FALLS IN FLOOR SURFACES ARE NOT PERMITTED UNLESS SHOWN ON DRAWINGS. MAINTAIN
- MINIMUM SLAB THICKNESS SHOWN ON PLAN WHERE FALLS OCCUR. **C13.** REINFORCEMENT QUALITY AND NOTATION

REINFORCEMENT NOTATION							
SYMBOL	BAR TYPE	STRENGTH GRADE (MPa)	DUCTILITY CLASS	TO COMPLY WITH AUSTRALIAN STANDARD			
S	STRUCTURAL GRADE DEFORMED RIB BAR	250	NORMAL	AS/NZS 4671-2001			
Ν	HOT ROLLED DEFORMED RIB BAR	500	NORMAL	AS/NZS 4671-2001			
R	PLAIN ROUND BAR	250	NORMAL	AS/NZS 4671-2001			
RL	RECTANGULAR MESH OF DEFORMED RIB BAR	500	LOW	AS/NZS 4671-2001			
SL	SQUARE MESH OF DEFORMED RIB BAR	500	LOW	AS/NZS 4671-2001			
L-TM	TRENCH MESH	500	LOW	AS/NZS 4671-2001			

ALL REINFORCING BARS SHALL BE GRADE D500N TO AS/NZS 4671-2001 AND ALL MESH SHALL BE GRADE 500L TO AS/NZS 4671-2001. UNLESS NOTED OTHERWISE CLASS L REINFORCEMENT SHALL NOT BE USED.

3/N20 - BAR SIZE (mm) — TYPE OF REINFORCEMENT NUMBER OF BARS

SL92 - BAR SPACING IN 100mm

- BAR SIZE (mm) - DUCTILITY CLASS - SQUARE MESH

ALL SETOUT TO ARCHITECT'S DRAWINGS DIMENSIONS TO BE VERIFIED WITH ARCHITECT AND BUILDER BEFORE COMMENCING SHOP DRAWINGS OR SITE WORK, NORTHROP ACCEPTS NO RESPONSIBIL FOR THE USABILITY, COMPLETENESS OR SCALE O DRAWINGS TRANSFERRED ELECTRONICALLY.

PROJECT SUBBASE PLATYPL 120 High St, North Sydne 2060

BOTH BOTTO BLOCK BRICK CENTRA CENTR CENTRE CIRCUL SECTIO CONCR CONCR CONST DEEP/ DIAME DIAME DIAME DIAME DRAWI EACH EACH EQUAL EQUAL EXISTI EXPAN FAR SI FAR F FINISHE FLAT GALVA GENER. BRACK HIGH/H HORIZ IMPOSI (LIVE INTERS LONG/ LENGT

CONCRETE (CONTINUED)

SHOWN ARE INDICATIVE ONLY AND LENGTHS MAY VARY. BEAM ELEVATIONS TAKE PRECEDENCE OVER SECTIONS. SLAB PLANS TAKE PRECEDENCE OVER SECTIONS. REFER TO SECTIONS FOR EXTRA BARS THAT MAY BE REQUIRED.

C15. USE ONLY PLASTIC OR CONCRETE CHAIRS AT EXTERNAL SURFACES.

C16. SITE BENDING OF REINFORCEMENT BARS SHALL BE DONE WITHOUT HEATING USING A RE-BENDING TOOL. THE BARS SHALL BE RE-BENT AGAINST A FLAT SURFACE OR A PIN WITH A DIAMETER NOT LESS THAN THE MINIMUM PIN SIZE PRESCRIBED IN AS3600-2018.

C17. SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN POSITIONS SHOWN ON THE STRUCTURAL DRAWINGS OR IN POSITIONS OTHERWISE APPROVED IN WRITING BY NORTHROP CONSULTING ENGINEERS. LAPS SHALL NOT BE LESS THAN THE DEVELOPMENT LENGTH FOR EACH BAR AND IN ACCORDANCE WITH AS3600-2018 SECTION 13. **C18.** LAPS IN MESH SHALL BE IN ACCORDANCE WITH AS3600-2018 SECTION 13.

C19. WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THE STRUCTURAL DRAWINGS OR APPROVED BY NORTHROP CONSULTING ENGINEERS.

C20. AT EXTERNALLY EXPOSED SURFACES NO METALLIC ITEMS INCLUDING FORM BOLTS, FORM SPACERS, METALLIC BAR CHAIRS AND TIE-WIRE ARE TO BE PLACED IN THE COVER ZONE.

C21. ALL REINFORCEMENT, ANCHOR BOLTS AND OTHER CONCRETE INSERTS SHALL BE WELL SECURED IN POSITION AND INSPECTED BY NORTHROP CONSULTING ENGINEERS PRIOR TO PLACING CONCRETE. C22. HOLD DOWN BOLTS SHALL BE HOT DIPPED GALVANISED.

C23. U.N.O., ALL MASONRY ANCHORS INTO CONCRETE SHALL BE RAMSET TRUBOLTS (LONGEST VERSION) OR APPROVED EQUIVALENT. BOLTS SHALL BE GALVANISED WHERE THEY ARE ADJOINING NON FERROUS OR PREPAINTED MEMBERS. PROVIDE STAINLESS STEEL BOLTS FOR ALL EXTERNAL CONDITIONS, OR WHERE EXPOSED TO THE WEATHER.

C24. ALL CONCRETE MIXES SHALL BE DESIGNED BY A RECOGNISED TESTING LAB AND SUBMITTED FOR REVIEW BY NORTHROP CONSULTING ENGINEERS.

C25. ALL COMPRESSIVE STRENGTH TEST REPORTS SHALL BE SUBMITTED TO NORTHROP CONSULTING ENGINEERS FOR REVIEW.

C26. PROJECT CONTROL TESTING SHALL BE CARRIED OUT ON ALL CONCRETE IN ACCORDANCE WITH AS1379-2007. TEST CYLINDERS ARE TO BE KEPT ON SITE.

C27. CURING OF ALL CONCRETE IS TO BE ACHIEVED BY KEEPING SURFACES CONTINUOUSLY WET FOR A PERIOD OF 7 DAYS, UNLESS SPECIFIED OTHERWISE. APPROVED SPRAY ON CURING COMPOUNDS THAT COMPLY WITH AS3799-1998 MAY BE USED WHERE FLOOR FINISHES WILL NOT BE AFFECTED. POLYTHENE SHEETING OR WET HESSIAN MAY BE USED TO RETAIN CONCRETE MOISTURE WHERE PROTECTED FROM WIND AND TRAFFIC. CURING IS TO COMMENCE IMMEDIATELY AFTER CONCRETE PLACEMENT.

C28. FOR ELAPSED TIME BETWEEN THE WETTING OF THE MIX AND THE DISCHARGE OF THE MIX, REFER TO CONCRETE - ELAPSED DELIVERY TIMES NOTE.

C29. BUILDER TO ALLOW TO COMPLETE FURTHER GEOTECHNICAL TESTING TO ASSESS THE PRESENCE OF ACID SULFATE SOILS ACROSS THE SITE. (REFER COUNCIL REQUIREMENTS).

IF ACID SULFATE SOILS ARE FOUND, ALLOW TO INCREASE CONCRETE STRENGTH OF CONCRETE IN CONTACT WITH GROUND TO 50MPa AND PROVIDE 2 LAYERS OF 0.2mm POLYTHENE MEMBRANE OF HIGH IMPACT RATING UNDER BUILDING SLABS.

LEGEND						
TERM	ABBREVIATION	TERM	ABBREVIATION			
OTH SIDES	B/S	MAXIMUM	MAX			
OTTOM	В	MINIMUM	MIN			
LOCK WALL	BW	MISCELLANEOUS	MISC			
RICK WALL	BRW	NEAR FACE	NF			
ENTRALLY PLACED	CENT	NEAR SIDE	N/S			
ENTRE LINE	CL	NOMINAL	NOM			
ENTRES	CTS	NOT SHOWN ON PLAN	NSOP			
IRCULAR HOLLOW ECTION	CHS	NOT SHOWN ON ELEVATION	NSOE			
ONCRETE	CONC	NOT TO SCALE	NTS			
ONCRETE WALL	CW	OPPOSITE	OPP			
ONSTRUCTION JOINT	CJ	OVERALL	0/A			
EEP/DEPTH	D	PARALLEL	PFC			
IAMETER	DIA	FLANGE CHANNEL				
IAMETER INSIDE	ID	PLATE	PL			
IAMETER NOMINAL	DN	PERMANENT ACTION	G			
IAMETER OUTSIDE	OD	(DEAD LUAD)	DT			
RAWING	DWG	PUST TENSION	PI			
ACH FACE	EF					
ACH WAY	EW		RAU			
QUAL	EQ		RHS			
QUAL ANGLE	EA		рг			
XISTING	EXST		DEINE			
XPANSION JOINT	EJ					
AR SIDE	F/S		REGO			
AR FACE	FF	SUUARE	Su			
INISHED FLOOR LEVEL	. FFL	SUUARED HOLLOW SECTION	SHS			
LAT	FL	TOP	т			
ALVANISED	GALV	TOP & BOTTOM	T&B			
ENERAL PURPOSE	GPB		TYP			
RACKET			11/5			
IGH/HEIGHT	Н		114			
ORIZONTAL	HORIZ	UNIVEDSAL REAM				
1POSED ACTION	Q					
IVE LUAD)						
NIERSELTIUN PUINT	IP I	OTHERWISE	UNU			
	L	VERTICAL	VERT			
ENUTH VARIES	LV	WIDE/WIDTH	W			

	DRAWING TITLE
JS ey NSW	STRUCTURAL DRAWING SPECIFICATION NOTES - SHEET 1

CONCRETE MATERIALS

ELIMIN SUED F	27 to 30 30 to 32 32 to 35 IF THE ELAPSED TIME IS LONGER THAN THE CC TEMPERATURE IS GREATER THAN 35°C, EITHER DESIGN ENGINEER ARE TO BE CONTACTED TO C POUR IS TO BE STOPPED. IF THE POUR IS STOP NORTHROP CONSULTING ENGINEERS ARE TO BE WHAT, IF ANY, RECTIFICATION WORKS ARE REC IF THE CONCRETE TEMPERATURE AT THE TIME REJECTED. IF AIR TEMPERATURE IS ≤ 10°C, (FOI CONCRETING PROCEDURES" FOR APPROVAL. DESCRIPTION ARY ISSUE OR TENDER TENDER ISSUF	1.50 1.00 0.75 0.50 RRESPONDING TIME IN THE TABLE ABOVE, OR THE NORTHROP CONSULTING ENGINEERS OR THE CONCRETE MORTHROP CONSULTING ENGINEERS OR THE CONCRETE PLACEMENT CONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE OPED, PRIOR TO ANY FURTHER CONCRETE PLACEMENT CONTACTED TO INSPECT THE WORKS AND DETERMINE UIRED. OF DISCHARGE IS NOT ≥ 5°C, CONCRETE SHALL BE R ≥ A 12 HOUR PERIOD) SUBMIT "COLD WEATHER ISSUED VER'D APP'D DATE HA RP HA RP HA RP 10.08.23
ELIMIN SUED F	27 to 30 30 to 32 32 to 35 IF THE ELAPSED TIME IS LONGER THAN THE CC TEMPERATURE IS GREATER THAN 35°C, EITHER DESIGN ENGINEER ARE TO BE CONTACTED TO C POUR IS TO BE STOPPED. IF THE POUR IS STOP NORTHROP CONSULTING ENGINEERS ARE TO BE WHAT, IF ANY, RECTIFICATION WORKS ARE REC IF THE CONCRETE TEMPERATURE AT THE TIME REJECTED. IF AIR TEMPERATURE IS ≤ 10°C, (FOI CONCRETING PROCEDURES" FOR APPROVAL. DESCRIPTION ARY ISSUE OR TENDER	1.50 1.00 0.75 0.50 RRESPONDING TIME IN THE TABLE ABOVE, OR THE NORTHROP CONSULTING ENGINEERS OR THE CONCRETE MONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE OPED, PRIOR TO ANY FURTHER CONCRETE PLACEMENT CONTACTED TO INSPECT THE WORKS AND DETERMINE UIRED. OF DISCHARGE IS NOT ≥ 5°C, CONCRETE SHALL BE R ≥ A 12 HOUR PERIOD) SUBMIT "COLD WEATHER ISSUED VER'D APP'D DATE HA RP HA RP HA RP
ELIMIN	24 to 27 27 to 30 30 to 32 32 to 35 IF THE ELAPSED TIME IS LONGER THAN THE CO TEMPERATURE IS GREATER THAN 35°C, EITHER DESIGN ENGINEER ARE TO BE CONTACTED TO C POUR IS TO BE STOPPED. IF THE POUR IS STOF NORTHROP CONSULTING ENGINEERS ARE TO BE WHAT, IF ANY, RECTIFICATION WORKS ARE REC IF THE CONCRETE TEMPERATURE AT THE TIME REJECTED. IF AIR TEMPERATURE AT THE TIME REJECTED. IF AIR TEMPERATURE IS ≤ 10°C, (FOI CONCRETING PROCEDURES" FOR APPROVAL.	1.50 1.00 0.75 0.50 RRESPONDING TIME IN THE TABLE ABOVE, OR THE NORTHROP CONSULTING ENGINEERS OR THE CONCRETE MONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE ONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE ONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE ONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE ONFIRM WHETHER PLACEMENT IS TO PROCEED OR IF THE ONFACTED TO INSPECT THE WORKS AND DETERMINE UIRED. OF DISCHARGE IS NOT ≥ 5°C, CONCRETE SHALL BE R ≥ A 12 HOUR PERIOD) SUBMIT "COLD WEATHER ISSUED VER'D APP'D DATE CLIENT HA RP 11.05.23
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	- 21.	2.00
	CONCRETE TEMPERATURE AT TIME OF DISCHARGE (°C)	MAXIMUM ELAPSED TIME (HOURS)
	ELAPSED DELIVERY	TIME TABLE
	NOT EXCEED THE CRITERIA IN THE ELAPSED DEL	IVERY TIMETABLE BELOW:
	UNLREIE (ELAPSED DELIV 1. ELAPSED TIME BETWEEN THE WETTING OF THE P	EKT TIMEST 11X AND THE DISCHARGE OF THE MIX AT THE SITE MUST
C	CONSIDERED A SEPARATE POUR FOR THE PURP	OSES OF TESTING.
	 IT IS PERMISSIBLE TO REDUCE THE NUMBER OF POURS OF LESS THAN 10m³, SAMPLE CONSISTS AT 7 DAYS. TWO CYLINDERS SHALL BE TESTED IMMEDIATELY TO NORTHROP CONSULTING ENGINE IF MORE THAN ONE STRENGTH GRADE IS BEING 	SAMPLES TO 2 FOR POURS OF LESS THAN 30m ³ , AND 1 OF AT LEAST 3 CYLINDERS. ONE CYLINDER SHALL BE T AT 28 DAYS. 7 & 28 DAY TEST RESULTS TO BE SENT ERS. USED IN A POUR, EACH STRENGTH GRADE SHALL BE
	SLUMP SPECIFIED. – AT LEAST 1 SAMPLE FOR EVERY 50m ³ SHALL E SHALL BE TAKEN PER POUR. FIRST AND LAST I PROGRESSIVELY DURING THE POUR, RECORDING	BE TAKEN AT THE SITE, BUT NOT LESS THAN 3 SAMPLE BATCH PER DAY TO BE SAMPLED, THE OTHER TAKEN LOCATION IN POUR WHERE SAMPLE WAS TAKEN.
	ASSESSMENT FOR THE GRADES OF CONCRETE SPE - CONCRETE SHALL BE SUPPLIED AND TESTED IN - THE ORGANISATION(S) RESPONSIBLE FOR SAMPL LABORATORY REGISTRATION, BE INDEPENDENT, AN OF SAMPLES AND SPECIMENS AND THE PREPARA - SI UMP AT TIME OF POURING TO BE WITHIN THE	CIFIED FOR THIS PROJECT. ACCORDANCE WITH AS1379-2007 AND AS1012. ING AND TESTING OF CONCRETE TO HAVE RELEVANT 'NA ID USE TRAINED, COMPETENT PERSONNEL FOR THE TAKII TION OF MATERIALS AND WORK FOR TESTING. PERMISSIBLE TOLERANCE IN AS1379-2007 FOR NOMINAL
СТ4.	PRODUCTION ASSESSMENT. CARRY OUT PROJECT ASSESSMENT FOR REMAINDE - SUPPLY OF CONCRETE SHALL BE FROM A CONC	R OF CONCRETE AS FOLLOWS : RETE BATCHING PLANT THAT CARRIES OUT PRODUCTION
СТ2. СТЗ.	PROVIDE EVIDENCE OF PRODUCTION ASSESSMENT CONCRETE SHRINKAGE AND CONCRETE FOR SLABS	TO AS1379-2007 PRIOR TO POURING. (UP TO 110mm THICK) ON GROUND, TO BE VERIFIED BY
CT1.	REQUIREMENT : - PRODUCTION ASSESSMENT - TESTING CARRI PROJECT ASSESSMENT - TESTING CARRI	ED OUT BY THE BATCHING PLANT.
102	CONTAINING CHLORIDES OR NITRATES PROH	IBITED.
CM8.	 NORMAL CLASS CONCRETE - RECYCLED WA CHEMICAL ADMIXTURES : TO AS 1478. USE IN ACCORDANCE WITH AS 	TER ACCEPTED. 1478 AND MANUFACTURER'S DIRECTIONS. ADMIXTURES
CM7.	ACCORDANCE WITH AS 11 . WATER : – SPECIAL CLASS CONCRETE – FROM A TOWI	41.14-2007. I POTABLE WATER SUPPLY.
	• PARTICLE SHAPE - TO AS2758.1 CLAUSE 9.3 USING A 2:1. RATIO IS NO	EQUIRED ON EACH SINGLE SOURCED AGGREGATE. AND THE PROPORTION OF MISSHAPEN PARTICLES T TO EXCEED 35% WHEN DETERMINED IN
	 WATER ABSORPTION - TO AS2758.1-1998 CL. DURABILITY - TO AS2758.1-1998 CL "SEVERE". ALKALI REACTIVITY - TEST FOR ALKALL REACTIVITY - TEST FOR ALKALL REACTIVITY 	AUSE 8.3, MAXIMUM 2.5% AUSE 10 CONCRETE EXPOSURE CLASSIFICATION- ACTIVE MATERIALS TO AS2758.1 CLAUSE 14.3
	GRADED AGGREGATE • PARTICLE DENSITY – TO AS2758.1–1998 CL • BULK DENSITY – TO AS2758.1–1998 CL	UF AS2758.1-1998. AUSE 8.1, MINIMUM 2100 kg/m ³ AUSE 8.3, MINIMUM 2100 kg/m ³
	 COARSE AGGREGATE – CLEAN, HARD, DURAB GRAVEL OR ROCK, CF BLENDED, CONFORMIN 	LE PARTICLES OF DENSE, NATURALLY OCCURRING USHED OR UNCRUSHED, AND EITHER SINGLE SOURCED OR 5 TO GRADING REQUIREMENTS OF 'TABLE 1', 20mm
	 STANDARD – TO AS2758.1–1998 FINE AGGREGATE – DENSE, NATURALLY OCCU EITHER SINGLE SOURCED OF 'TABLE 3', UNCRUSHE 	JRRING SAND OR ROCK, CRUSHED OR UNCRUSHED, AND OR BLENDED, CONFORMING TO GRADING REQUIREMENTS O FINE AGGREGATE OFAS2758.1–1998
CM6.	PERFORMANCE REQUIREME	INT OF THE CEMENT TYPE SPECIFIED.
2113.	 STANDARD - TO AS3972-1997 REQUIREMENT - ONLY WHEN SPECIFIED. A CONSIDERED, PROVIDED T 	LTERNATIVE MIX DESIGN WITH SILICA FUME WILL BE HE CEMENTITIOUS MATERIAL MEETS THE
rm5	REQUIREMENT - ONLY WHEN SPECIFIED. A CONSIDERED, PROVIDED T PERFORMANCE REQUIREMI SILICA FLIME ·	LTERNATIVE MIX DESIGN WITH FLY ASH WILL BE HE CEMENTITIOUS MATERIAL MEETS THE ENT OF THE CEMENT TYPE SPECIFIED.
	• REQUIREMENT - USE ONLY WHEN SPECIFIED. FLY ASH :	
CM4.	• STANDARD - TO AS3972-1997 BLENDED CEMENTS :	
CM3. CM4.		

CM1. CARRY OUT ALL CONCRETE WORK IN ACCORDANCE WITH AS3600-2018 AND NATSPEC CONCRETE STANDARDS

CHEMICAL ANCHORS

CHEMICAL INJECTION ANCHORS. CA02. ALTERNATIVE CHEMICAL ANCHORS MAY BE SUBSTITUTED WITH PRIOR PERMISSION FROM THE SUPERINTENDENT CA03. MINIMUM EDGE DISTANCE AND SPACING SETOUT OF THE ANCHORS ARE SPECIFIED ON THESE DRAWINGS. IF

INSTRUCTION.

CA06. CLEAN AND DEGREASE BOLT/BARS PRIOR INSTALLATION.

LOADING BOLTS/BARS.

CONCRETE MASONRY

CM1. MASONRY CONSTRUCTION IS TO CONFORM TO AS3700-2001. - MORTAR CLASSIFICATION

- DURABILITY CLASSIFICATION OF BUILT IN COMPONENTS = R4

- DURABILITY GRADE OF EXTERNAL MASONRY UNITS = EXPOSURE

CM2. THE CHARACTERISTIC UNCONFINED COMPRESSIVE STRENGTH OF THE MASONRY UNITS SHALL BE 15MPa OR GREATER.

1:1:6 or 1:0:5 CONFORMING TO AS3700-2001.

CM4. GROUT FILL FOR BLOCKWORK:

- COMPRESSIVE STRENGTH

- MAXIMUM AGGREGATE SIZE

– SLUMP – MINIMUM PORTLAND CEMENT CONTENT = 300kg/m³

SLABS

CM5. CONTROL JOINTS IN UNREINFORCED WALLS SHALL BE PROVIDED AS FOLLOWS; CLASS A & S - 5m MAX CTS

CLASS M - 5m MAX CTS (UP TO 4m HIGH WALL), 3.9m MAX CTS (4.0m to 8.5m HIGH WALL) CLASS H – 4.5m MAX CTS (UP TO 4m HIGH WALL), 3.2m MAX CTS (4.0m to 8.5m HIGH WALL)

- JOINTS TO BE 0.47m MINIMUM FROM CORNERS
- JOINT TO BE 4.5m MAXIMUM FROM CORNERS
- WHERE THE HEIGHT OF THE WALL CHANGES BY MORE THAN 20%, AT THE POSITION OF THE CHANGE - WHERE THE WALL CHANGES IN THICKNESS
- AT CONTROL OR CONSTRUCTION JOINTS IN SLABS

- AT JUNCTIONS OF WALLS CONSTRUCTED OF DIFFERENT MASONRY MATERIAL CM6. PROVIDE SLIDING HORIZONTAL TIES ACROSS JOINTS IN UNREINFORCED WALLS EQUIVALENT TO M.E.T 3-3

AT 400 CTS VERTICALLY IN EACH FACE OF THE BLOCKS.

- (600 LONG) DOWELS, PAINT ONE END WITH BITUMEN & PROVIDED EXPANSION CAP. **CM8**. THE BOTTOM COURSE OF ALL REINFORCED BLOCKWORK SHALL CONSIST OF E-SHAPED BLOCKS TO ENABLE
 - CLEANOUT AND TYING OF STEEL.
 - FULLY BED FACE SHELLS ONLY.

- CLEAN OUT ALL CORES AFTER EACH DAY'S LAYING. - ENSURE STARTER BARS ARE CORRECTLY LOCATED IN FOOTINGS.

- CM9. MASONRY TIES SHALL BE HOT DIP GALVANISED WITH MINIMUM COATING MASS OF 300g/m² AND MEDIUM DUTY CLASSIFICATION FOR CAVITIES UP TO 60mm WIDE & HEAVY DUTY FOR CAVITIES OVER 60mm WIDE. ANY FACE FIXED TIES SHALL BE FIXED USING A SCREW FIXING AND SHALL NOT BE NAILED.
- CM10. THE TOP COURSE OF ALL FREESTANDING HOLLOW BLOCK MASONRY SHALL CONSIST OF SOLID CAPPING BLOCKS.
- CM11. SPACING OF MASONRY TIES:

 - SOLID MASONRY
 - OTHERWISE
- FASTENED TO THE SOFFIT USING M.E.T 4 SLIDING TIES OR APPROVED EQUIVALENT AT 400mm CENTRES U.N.O.
- CM13. LOADBEARING HOLLOW BLOCK WALLS SHALL BE CAPPED WITH M.E.T. GRAPHITE GREASED SLIP JOINT OVER TOP COURSE OF BLOCKWORK.
- CM14. WHERE MASONRY ADJOINS STRUCTURAL STEEL OR PASSES A RETURN WALL ON THE INNER SKIN, INSTALL MEDIUM DUTY TIES @ 400 MAXIMUM CT. SHOT FIX TIES TO STEELWORK.
- CM15. MINIMUM COVER TO REINFORCEMENT FROM THE INSIDE FACE OF THE FACE SHELL IS TO BE 25mm. CM16. NO AIR ENTRAINING AGENTS (BYCOL, ETC.) ARE TO BE USED WITHOUT PRIOR WRITTEN PERMISSION FROM
- NORTHROP CONSULTING ENGINEERS. CM17. MATERIALS INCLUDING MORTAR, CONCRETE, GROUT SHALL COMPLY WITH SECTION 10 OF AS3700-2001. MASONRY UNITS SHALL COMPLY WITH AS/NZS 4455.1-2008. WALL TIES SHALL COMPLY WITH AS/NZS
- 2699.1-2000. CM18. MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 11 OF AS3700-2001.
- CM19. DO NOT CONSTRUCT UNREINFORCED OR NON-LOAD BEARING REINFORCED MASONRY WALLS ON SUSPENDED CONCRETE SLABS UNTIL SLAB HAS BEEN STRIPPED AND DE-PROPPED. CM20. ALL LINTELS SUPPORTING BLOCKWORK ARE TO BE HOT DIP GALVANISED WITH MINIMUM COATING MASS OF 300q/m².
 - PROVIDE 1 LINTEL TO EACH WALL LEAF. DO NOT CUT ON SITE. KEEP LINTELS 6mm CLEAR OF DOOR HEADS AND WINDOW FRAMES. PACK MORTAR BETWEEN THE ANGLE UPSTAND AND SUPPORT MASONRY UNITS. MINIMUM BEARING EACH END OF LINTELS
 - SPANS 0mm to 1800mm = 200mm BEARING EACH END. - SPANS 1801mm to 3000mm = 400mm BEARING EACH END. PROPPING OF LINTELS:

DAYS.

REV	DESCRIPTION	ISSUED	VER'D	APP'D	DATE	CLIENT	ARCHITECT
1	PRELIMINARY ISSUE	HA		RP	11.05.23		
2	ISSUED FOR TENDER	HA		RP	24.05.23		
3	REVISED TENDER ISSUE	HA		RP	10.08.23	IAILUK	
4	REVISED TENDER ISSUE	NTH		RP	08.09.23		
5	AS BUILT	TAT		RP	21.02.24		
						VERIFICATION SIGNATURE HAS BEEN ADDED	CONSULT
A							

CA01. UNLESS NOTED OTHERWISE, CHEMICAL ANCHORS SPECIFIED IN THESE DRAWINGS REFER RAMSET REO 502

- THE INSTALLED DISTANCES ARE LESS THAN THAT SPECIFIED NOTIFY THE SUPERINTENDENT FOR
- CA04. CHEMICAL ANCHORS ARE TO BE STIRCTLY INSTALLED TO MANUFACTURERS INSTALLATION PROCEDURE. CA05. DIAMETER OF HOLES TO MANUFACTURES SPECIFICATION FOR NOMINATED BOLT/BAR DIAMETER. DRILL HOLES USING A ROTARY PERCUSSION DRILL. DO NOT CORE DRILL HOLES.
- CA07. ENSURE CHEMICAL IS ALLOWED TO FULLY CURE IN ACCORDANCE WITH MANUFACTURE'S DETAILS PRIOR TO

 - = M4
- CM3. BED UNITS IN FRESHLY PREPARED MORTAR, UNIFORMLY MIXED CEMENT, LIME AND SAND IN THE RATIO OF
 - = N20MPa
 - = 10mm
 - = 225mm
 - COMPACT THE GROUT USING A MECHANICAL VIBRATOR AT CONTROL OR CONSTRUCTION JOINTS IN
- CM7. CONTROL JOINTS IN REINFORCED WALLS SHALL BE PROVIDED AT 12.0m CENTRES, PROVIDE R16-400

 - ADJACENT TO WINDOWS AND RETURN WALLS = 400mm VERTICAL AND HORIZONTAL
 - = 400mm VERTICAL AND HORIZONTAL
- = 800mm VERTICAL AND HORIZONTAL CM12. NON-LOADBEARING HOLLOW BLOCK WALLS SHALL FINISH 20mm SHORT OF SLAB SOFFIT AND SHALL BE

TO PREVENT DEFLECTION OR EXCESSIVE ROTATION, TEMPORARILY PROP PROPRIETARY COLD-FORMED LINTELS UNTIL THE MASONRY REACHES ITS REQUIRED STRENGTH. MINIMUM PROPPING PERIOD IS TO BE 7

WORKPLACE HEALTH AND SAFETY

- WHS1. THE CONTRACTOR AND ALL SUBCONTRACTORS ARE RESPONSIBLE FOR CONSTRUCTING THE WORK IN ACCORDANCE WITH THE WORK HEALTH AND SAFETY (WHS) ACT 2011; WHS REGULATIONS 2017; RELEVANT CODES OF PRACTICE, AUSTRALIAN STANDARDS AND OTHER REGULATORY REQUIREMENTS. THE PRINCIPLE CONTRACTOR MUST INFORM ALL STAKEHOLDERS, INCLUDING NORTHROP, OF NEW HAZARDS IDENTIFIED IN THE COURSE OF PLANNING AND UNDERTAKING THE WORKS. WHS2. DURING THE DESIGN OF THE STRUCTURE NORTHROP HAS IDENTIFIED RESIDUAL HAZARDS RELATING TO
- THE DESIGN OF THE STRUCTURAL WORKS THAT WE CONSIDER TO BE UNUSUAL OR NON-TYPICAL. HAZARDS WHICH ARE NORMAL WORKPLACE HAZARDS, ARE TO BE MANAGED BY PERSONS IN CONTROL OF THE WORKPLACE THROUGH A WHS SYSTEM TO MANAGE THE NORMAL HAZARDS ASSOCIATED WITH CONSTRUCTION, USE AND MAINTENANCE OF THE STRUCTURE. THE RESIDUAL HAZARDS IDENTIFIED ON THE NORTHROP DRAWINGS ARE NOT AN ENTIRE ASSESSMENT OF HAZARDS, AND DO NOT RELIEVE OTHER PARTIES OR STAKEHOLDERS OF THE THEIR RESPONSIBILITY UNDER THE WHS ACT 2011, WHS REGULATIONS 2017, AND THE CODE OF PRACTICE FOR SAFE DESIGN OF STRUCTURES.
- WHS3. TEMPORARY BRACING AND SUPPORT OF STRUCTURE IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL BE MAINTAINED DURING ALL STAGES OF CONSTRUCTION.
- WHS4. RESIDUAL HAZARDS ARE SHOWN ON THE NORTHROP DRAWINGS. IDENTIFIED BY: S1

CLAY BRICK MASONRY

- CB1. MASONRY CONSTRUCTION IS TO CONFORM TO AS3700-2018.
 - MORTAR CLASSIFICATION = M4 - DURABILITY CLASSIFICATION OF BUILT IN COMPONENTS = R4
 - DURABILITY GRADE OF EXTERNAL MASONRY UNITS = EXPOSURE
- CB2. THE CHARACTERISTIC UNCONFINED COMPRESSIVE STRENGTH OF THE MASONRY UNITS SHALL BE A MINIMUM OF 20MPa.
- CB3. BED UNITS IN FRESHLY PREPARED MORTAR, UNIFORMLY MIXED CEMENT, LIME AND SAND IN THE RATIO OF 1:0.5:4.5 CONFORMING TO AS 3700-2011.
- CB4. NO CUTTING, CHASING OR RAKING OF JOINTS IN EXCESS OF 5mm UNLESS AUTHORISED BY NORTHROP
- CONSULTING ENGINEERS FOR EVERY LOCATION.
- **CB5.** CONTROL JOINTS IN UNREINFORCED WALLS SHALL BE PROVIDED AS FOLLOWS;
 - CLASS A & S 8m MAX CTS & 5m CTS WHERE OPENINGS ≥ 0.9m x 0.9m OCCUR
 - CLASS M 5m MAX CTS (UP TO 4m HIGH WALL), 3.9m MAX CTS (4.0m to 8.5m HIGH WALL) CLASS H - 4.5m MAX CTS (UP TO 4m HIGH WALL), 3.2m MAX CTS (4.0m to 8.5m HIGH WALL)
 - JOINTS TO BE 0.47m MINIMUM FROM CORNERS
 - JOINTS TO BE 4.5m MAXIMUM FROM CORNERS
 - JOINTS TO BE 4.0m MAXIMUM CTS AT PARAPETS
 - WHERE THE HEIGHT OF THE WALL CHANGES BY MORE THAN 20%, AT THE POSITION OF THE CHANGE - WHERE THE WALL CHANGES IN THICKNESS
 - AT CONTROL OR CONSTRUCTION JOINTS IN SLABS
 - AT JUNCTIONS OF WALLS CONSTRUCTED OF DIFFERENT MASONRY MATERIAL
- CB6. MASONRY TIES SHALL BE GRADE 316 STAINLESS STEEL AND OF MEDIUM DUTY CLASSIFICATION FOR CAVITIES UP TO 60mm WIDE & HEAVY DUTY CLASSIFICATION FOR CAVITIES OVER 60mm WIDE. ANY FACE FIXED TIE SHALL BE FIXED USING A SCREW TYPE FIXING AND SHALL NOT BE NAILED.
- CB7. PROVIDE SLIDING HORIZONTAL TIES ACROSS JOINTS IN WALLS EQUIVALENT TO MET 3-3 AT 460 CTS VERTICALLY IN EACH FACE OF THE BRICKS.
- **CB8.** SPACING OF MASONRY TIES:
 - 460mm VERTICALLY & 470mm HORIZONTALLY ADJACENT TO WINDOWS AND RETURN WALLS. – 460mm VERTICALLY & 470mm HORIZONTALLY FOR SOLID MASONRY
- OTHERWISE, 600mm VERTICALLY AND HORIZONTALLY. CB9. WHERE MASONRY ADJOINS STRUCTURAL STEEL, OR PASSES A RETURN WALL ON THE INNER SKIN
- PROVIDE MEDIUM DUTY TIES AT 360mm MAXIMUM CENTRES. SHOT FIX TIES TO STEELWORK. CB10. LOADBEARING WALLS SHALL BE TOPPED WITH TWO (2) LAYERS OF ALCOR OVER THE TOP COURSE OF BRICKWORK, TROWEL A LAYER OF MORTAR FILLING ALL HOLES AND DEPRESSIONS IN TOP COURSE OF
- BRICKWORK UNDER THE ALCOR. CB11. NON-LOADBEARING WALLS SHALL FINISH 20mm SHORT OF SLAB SOFFIT AND SHALL BE FASTENED TO
- THE SLAB SOFFIT USING M.E.T 4 SLIDING TIES OR APPROVED EQUIVALENT AT 460mm CENTRES. CB12. U.N.O. ALL MASONRY ANCHORS INTO CLAY MASONRY SHALL BE HILTI HIT-HY270 CHEMICAL INJECTION
- ANCHORS OR APPROVED EQUIVALENT & ANCHORS EXPOSED TO WEATHER SHALL BE GRADE 316 STAINLESS STEEL CB13. NO AIR ENTRAINING AGENTS (BYCOL, ETC) ARE TO BE USED WITHOUT PRIOR WRITTEN PERMISSION FROM
- NORTHROP CONSULTING ENGINEERS.
- CB14. MASONRY SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION 11 OF AS3700.
- CB15. DO NOT CONSTRUCT MASONRY WALLS ON SUSPENDED CONCRETE SLABS UNTIL SLAB HAS BEEN STRIPPED AND DEPROPPED.
- CB16. ALL LINTELS SUPPORTING BRICKWORK ARE TO BE GRADE 316 STAINLESS STEEL or R4 RATED LINTEL. PROVIDE 1 LINTEL TO EACH WALL LEAF. DO NOT CUT ON SITE. KEEP LINTELS 6mm CLEAR OF DOOR HEADS AND WINDOW FRAMES. PACK MORTAR BETWEEN THE ANGLE UPSTAND AND SUPPORT MASONRY UNITS. MINIMUM LINTEL SIZES (AS PER AS4773.1) OR AS FOLLOWS:
 - SPANS 0mm to 900mm = 10mm FLAT BAR.
 - SPANS 901mm to 2100mm = 100 x 100 x 10 EA.
 - SPANS 2101mm to 3100mm = 150 x 100 x 10 UA.
 - MINIMUM BEARING EACH END OF LINTELS:
 - SPANS 0mm to 1800mm = 110mm BEARING EACH END. - SPANS 1801mm to 3000mm = 230mm BEARING EACH END.
 - PROPPING OF LINTELS:

TO PREVENT DEFLECTION OR EXCESSIVE ROTATION, TEMPORARILY PROP PROPRIETARY COLD-FORMED LINTELS UNTIL THE MASONRY REACHES ITS REQUIRED STRENGTH. MINIMUM PROPPING PERIOD IS TO BE 7 DAYS.

ALL SETOUT TO ARCHITECT'S DRAWINGS DIMENSIONS TO BE VERIFIED WITH ARCHITECT AND BUILDER BEFORE COMMENCING SHOP DRAWINGS OR SITE WORK, NORTHROP ACCEPTS NO RESPONSIBILI FOR THE USABILITY, COMPLETENESS OR SCALE O DRAWINGS TRANSFERRED ELECTRONICALLY.

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10.

8.

PAVEMENT JOINTS

- PROVIDE ISOLATION JOINTS BETWEEN NEW CONCRETE WORKS AND EXISTING STRUCTURES. PAVEMENTS.
- ALL PAVEMENTS ARE TO BE JOINTED AS FOLLOWS
- U.N.O. ON THE DESIGN DRAWINGS. DOWELLED EXPANSION JOINTS ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES AND ELSEWHERE AT MAX.
- CENTRES AS PER DETAIL.

2.

- SAWN JOINTS ARE TO BE LOCATED AT A MAX. SPACING OF
- 1.5m x WIDTH OF THE PAVEMENT. WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND OR ADJACENT PAVEMENT JOINTS.
- TYPICAL PAVEMENT JOINT DETAIL TYPE A.

7. TYPICAL PAVEMENT JOINT DETAIL TYPE B.

KERB EXPANSION JOINTS SHALL BE FORMED 10mm ABLEFLEX FOR FULL DEPTH OF SECTION.

KERB EXPANSION JOINTS TO BE LOCATED AR DRAINAGE PITS, TANGENT POINTS OF CURVES / CORNERS AND AT 12m MAX CENTRES. KERB TOOLED JOINTS TO BE MIN 3mm WIDE AND LOCATED AT MAX 3m CENTRES.

INTEGRAL KERB JOINTS SHALL MATCH THE LOCATION OF PAVEMENT JOINTS.

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VERIFICATION SIGNATURE HAS BEEN ADDED

WALL SCHEDULE				
(THICKNESS	COMMENT(S)		
	190	BRICK WALL. N16-400 DOWELS DRILL & EPOXY ON ANGLE INTO UNDERSIDE OF EXISTING FOOTING		
	200	50MPa REINFORCED CONCRETE – N12–200 EW EF.		

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I4 Registered Survey

Author	Client	Drawing/Title
turf	Harbour Trust	Leve
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Revision